

Brief Expansion of Post 27

Solved Example 4-14 From Prof. SEGUI's ^{BOOK}

A given Multi storey Frame with steel design
a given AB $W_{10} \times 33$ Column which is an inelastic Column
The Frame is unbraced Frame, Girders data
are given.

$D \}$
 $L \}$ → Loads acting are given. Find K Factor

Prepared by Eng. Maged Kamel.

LRFD
ASD

Do not multiply G For pinned and Fixed support by I_p

If the end of a column is fixed ($G = 1.0$) or pinned ($G = 10.0$), the value of G at that end should not be multiplied by the stiffness reduction factor. Values of the stiffness reduction factor τ_b as a function of P_u/A_g and P_u/A_g are given in Table 4-21 in Part 4 of the *Manual*.

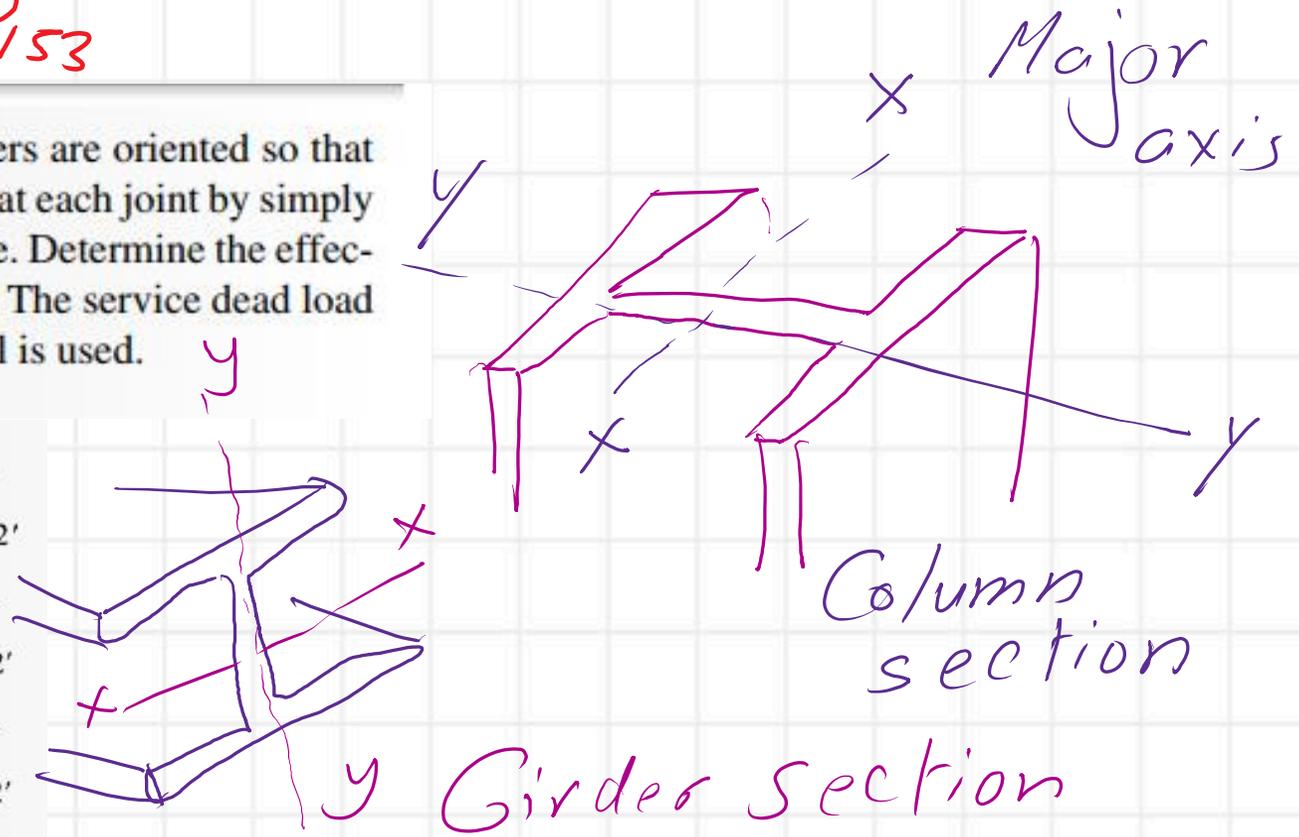
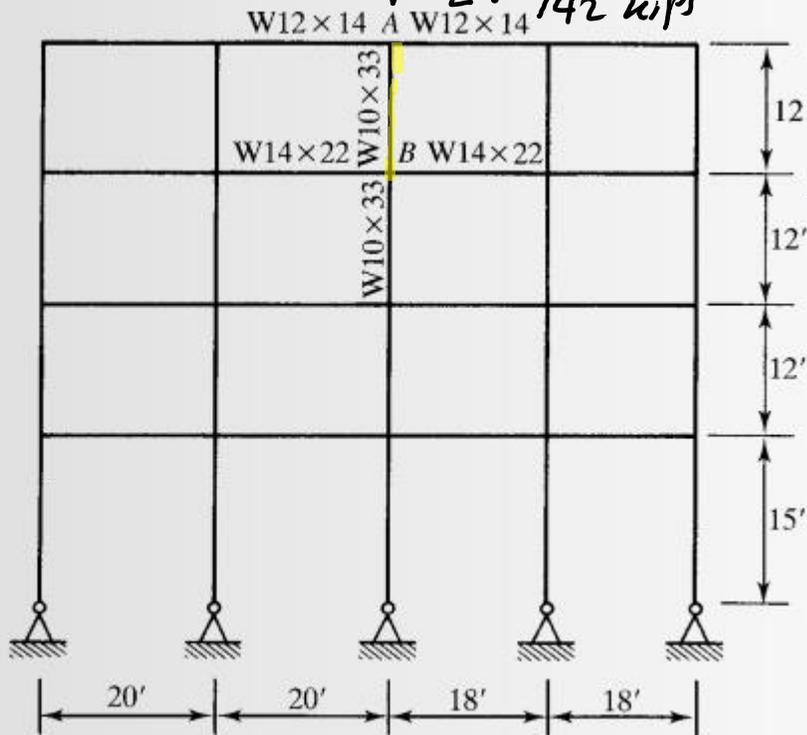
EXAMPLE 4.14

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A rigid unbraced frame is shown in Figure 4.17. All members are oriented so that bending is about the strong axis. Lateral support is provided at each joint by simply connected bracing in the direction perpendicular to the frame. Determine the effective length factors with respect to each axis for member AB. The service dead load is 35.5 kips, and the service live load is 142 kips. A992 steel is used.

$\downarrow D : 35.50 \text{ kip}$
 $L : 142 \text{ kips}$

FIGURE 4.17



$E = 29000 \text{ ksi}$
 A992 steel $F_y = 50 \text{ ksi}$

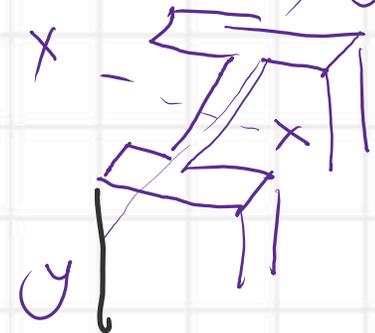
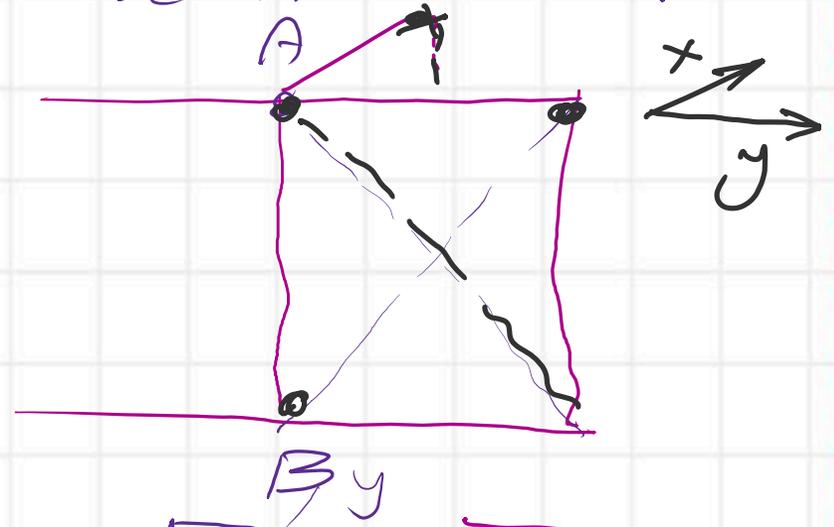
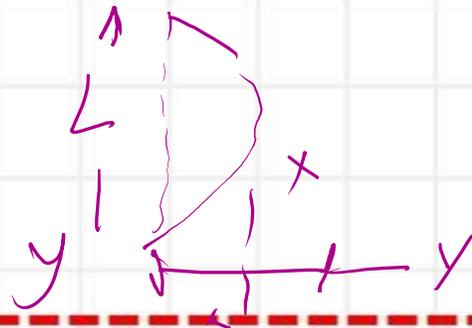
Check whether Column AB is Elastic or inelastic

$$\left(\frac{KL}{r}\right)_x = \left(\frac{le}{r}\right)_x \geq 4.71 \sqrt{\frac{E}{F_y}}$$

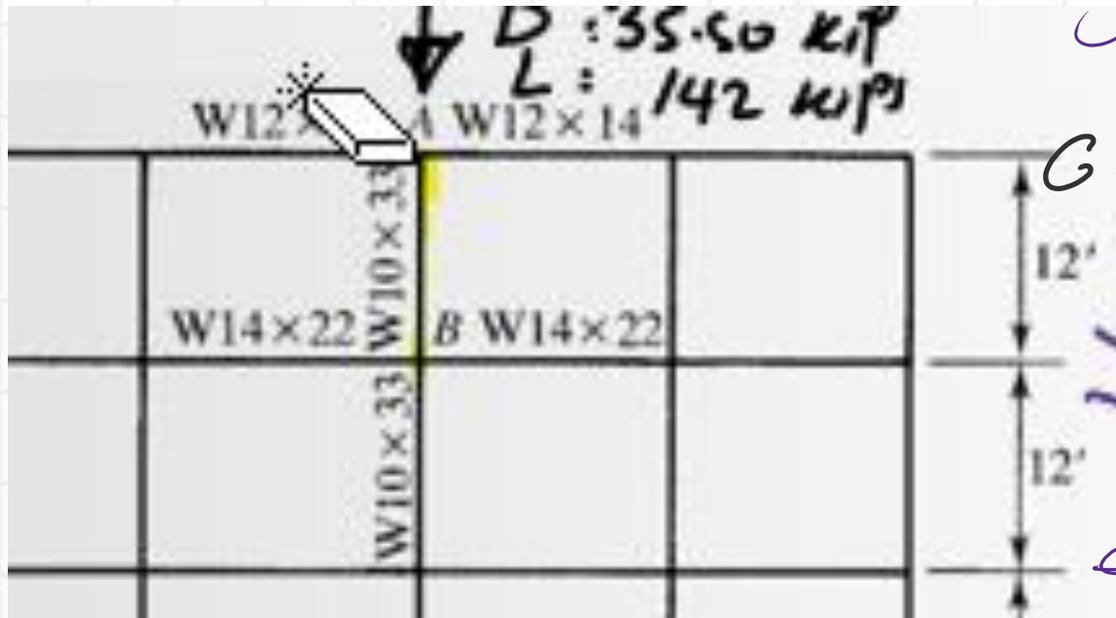
we need K value

Frame is braced in the y-direction

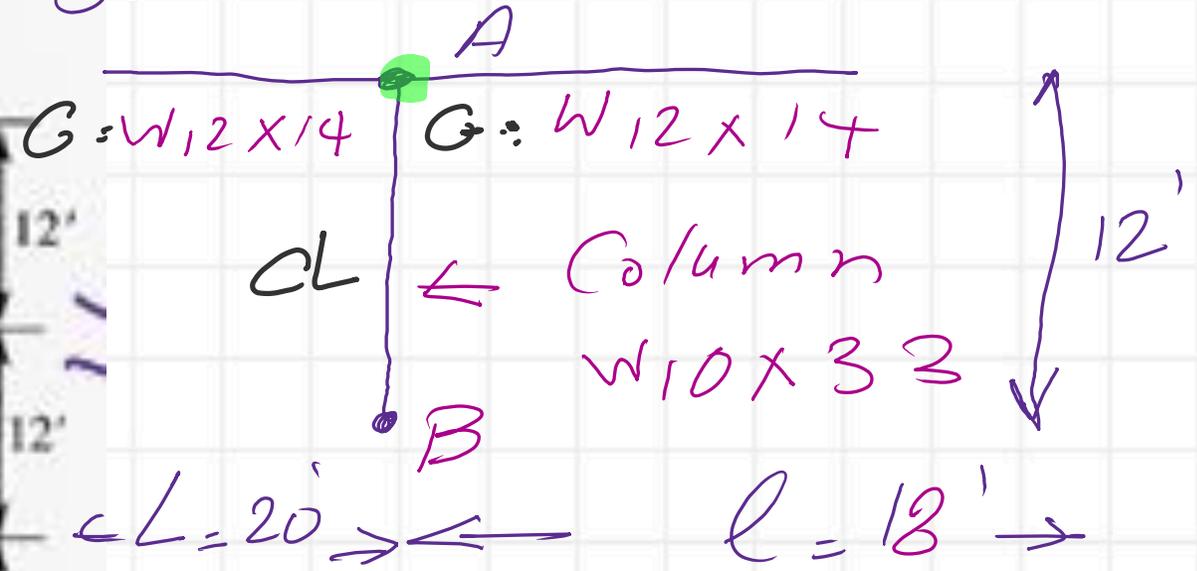
We do our calculation for x-direction



y-direction



Joint A



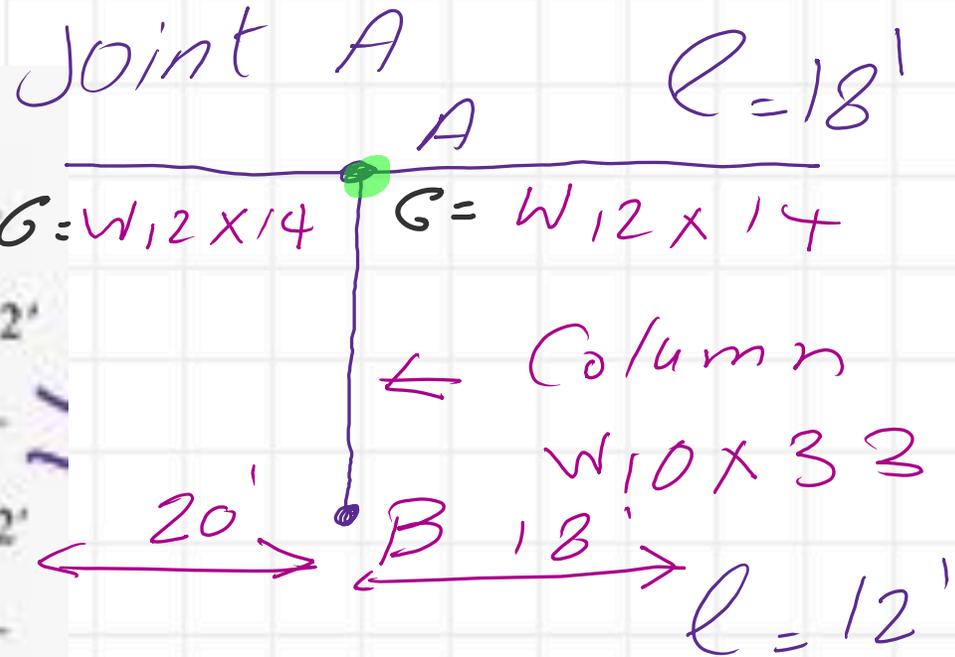
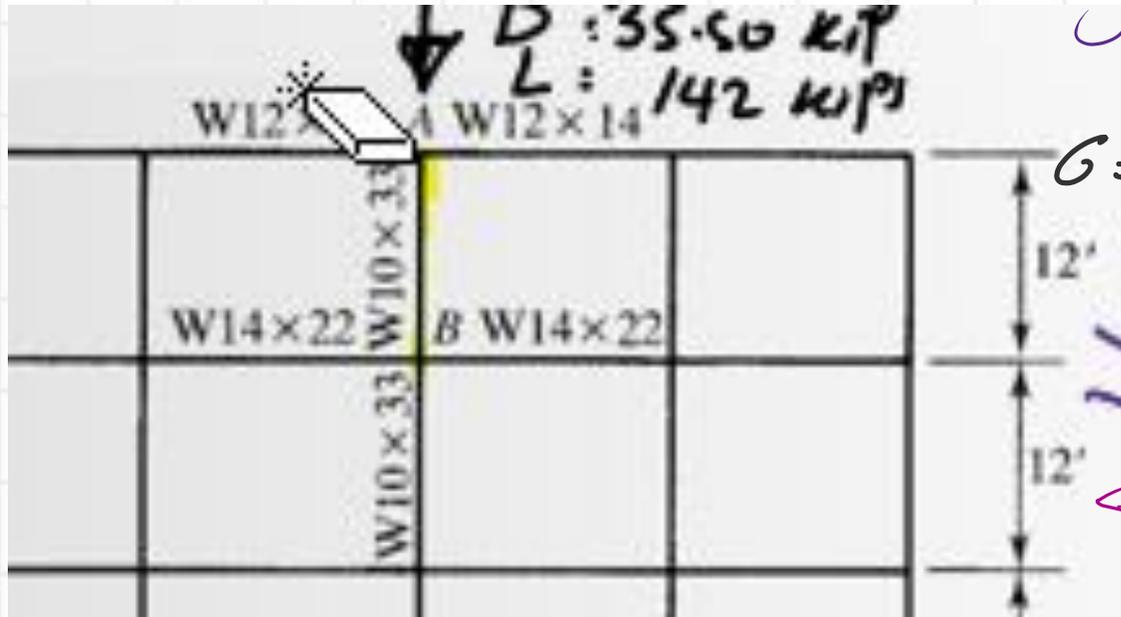
Type	Std_Nomen	AISC_Manual_La	Weigt	A
W	W10X33	W10X33	33.0	9.71

I_x	Z_x	S_x	r_x
171	38.8	35.0	4.19

Column AB

$$\left(\frac{EI_x}{L_x}\right)_C = E \left(\frac{171}{12(12)}\right)$$

$$\left(\frac{EI_x}{L_x}\right)_{\text{Column AB}} = E(171/44)$$



Two Girders (beams) $W12 \times 14$

Type	Std Nomen	AISC Manual La	Weigl	A
W	W12X14	W12X14	14.0	4.16

I_x	Z_x	S_x	r_x
88.6	17.4	14.9	4.62

$$G_A = \frac{E (171/12)}{E (3366.8/360)} \rightarrow \text{Column} = 1.5237$$

Column AB

$$\left(\frac{EI_x}{L_x}\right)_C = E \left(\frac{171}{12}\right)$$

$$\Sigma \left(\frac{EI}{L_x}\right)_{\text{Beam}} = E \left(\frac{88.6}{18} + \frac{88.6}{20}\right) \rightarrow E (3366.80/360)$$

Prepared by Eng.Maged Kamel.