

Topics

① How Can We Find the plastic Nominal Load P_n Value? Plastic Hinge Length.

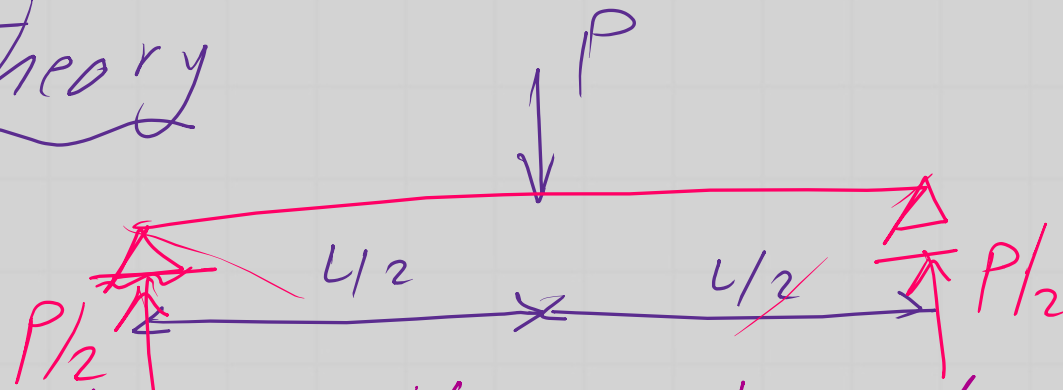
The Load is acting on the Middle of a simple beam → Two methods are Use : Statical method

② Determine P_n For a given W18x76 Beam A992 steel of 32 FT Long

Simple → Verify the Value by using MASTAN-2

Prepared by Eng. Maged Kamel.

Plastic Theory



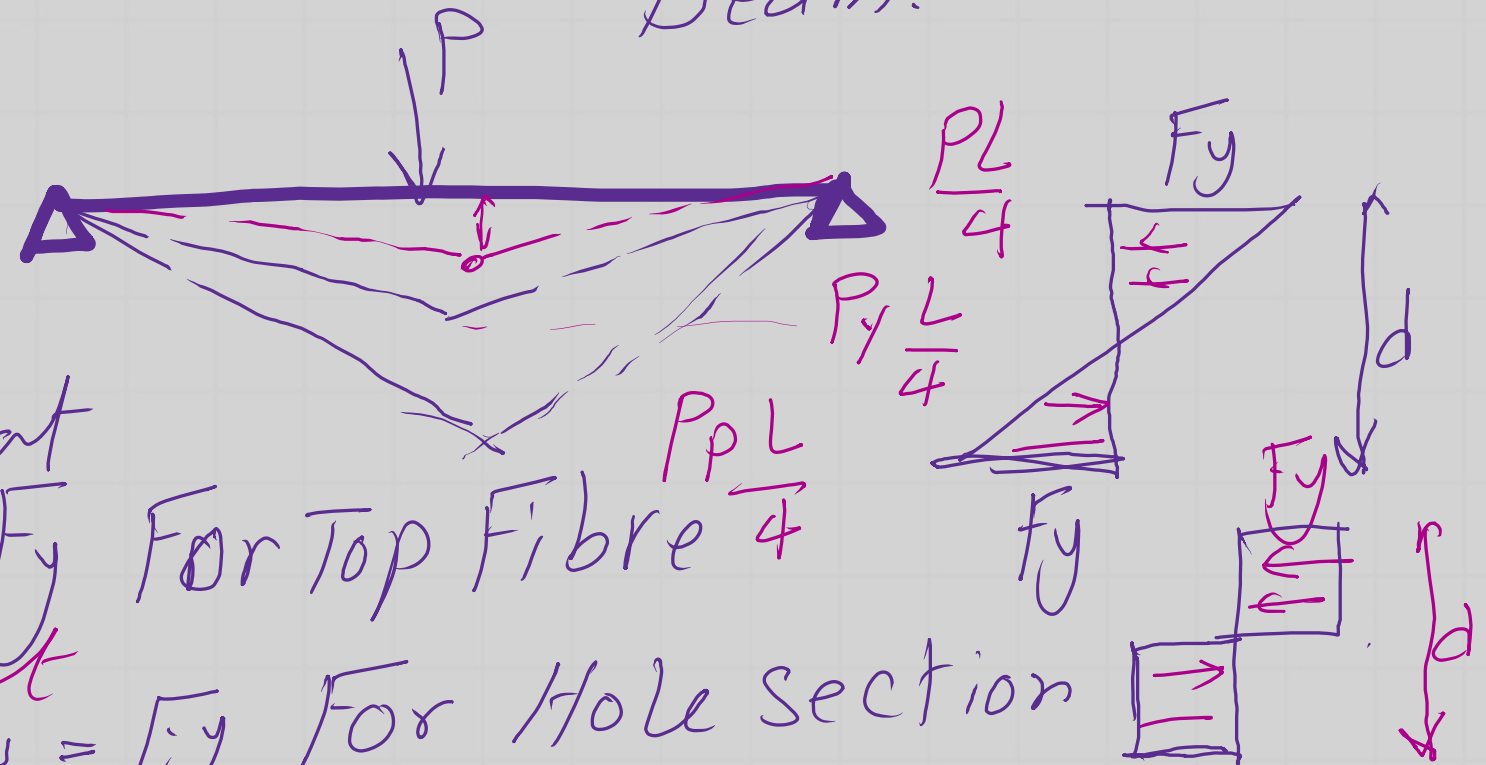
Beam of length L

Estimate the Collapse Load For the given Beam.

Load is

increasing

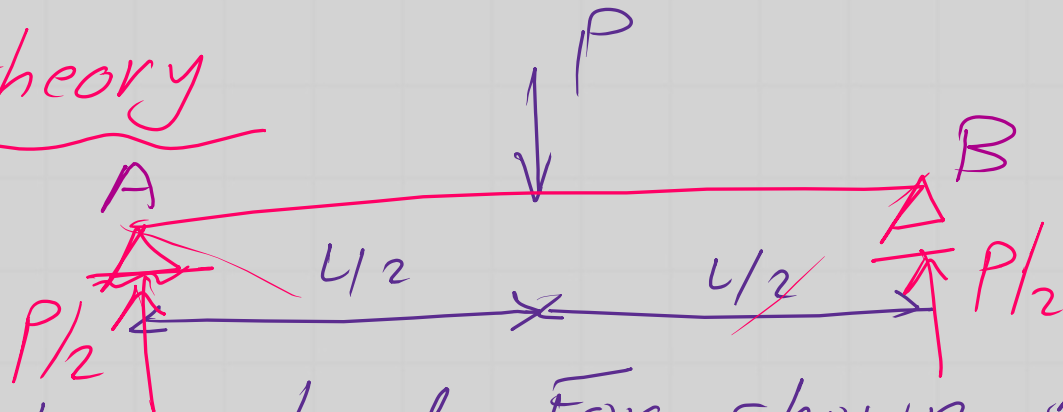
From $P \rightarrow P_y \rightarrow P_p$



P_y : The Load that Causes stress = F_y For Top Fibre $\frac{P_y L}{4}$

P_p : The Load that Causes stress = F_y For Hole section $\frac{P_p L}{4}$

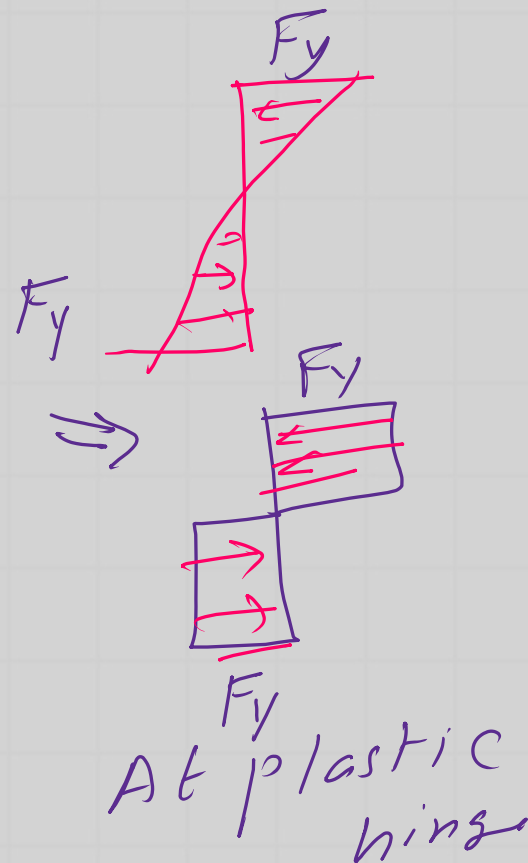
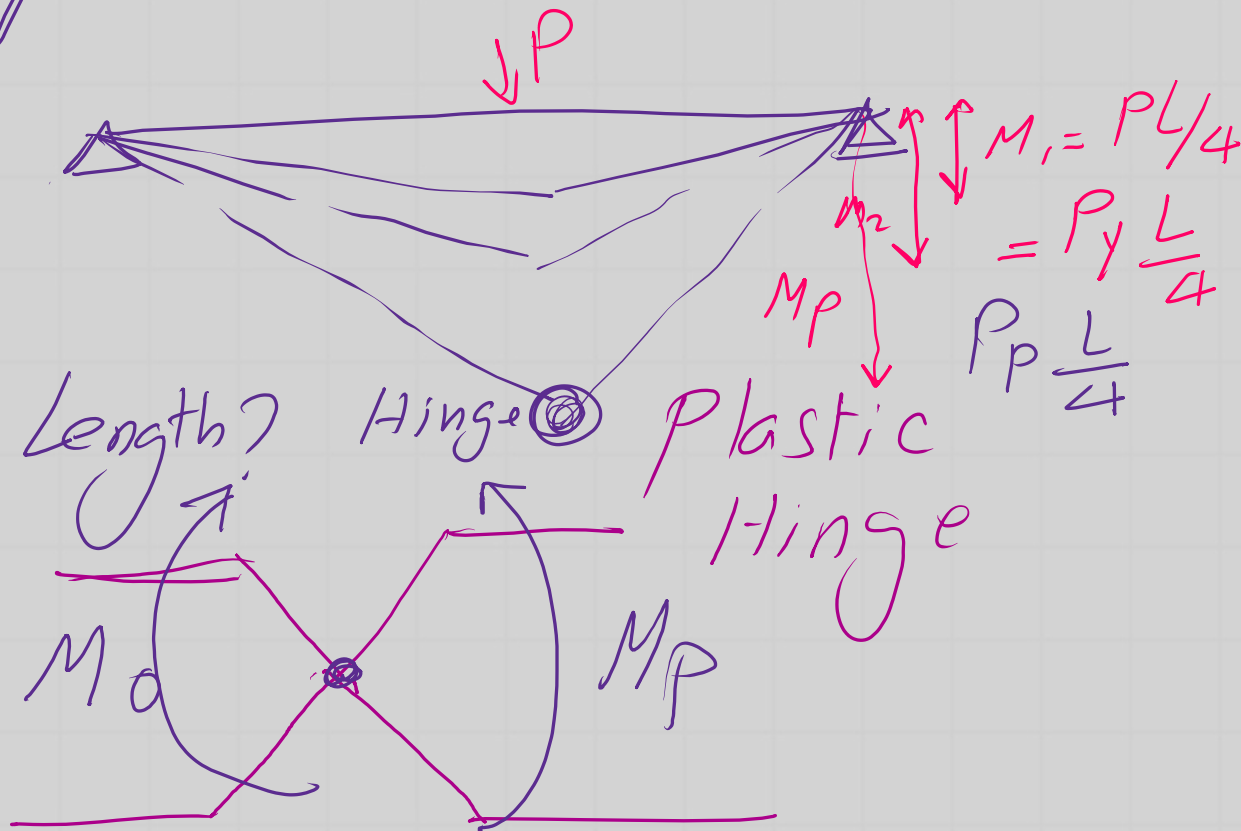
Plastic theory



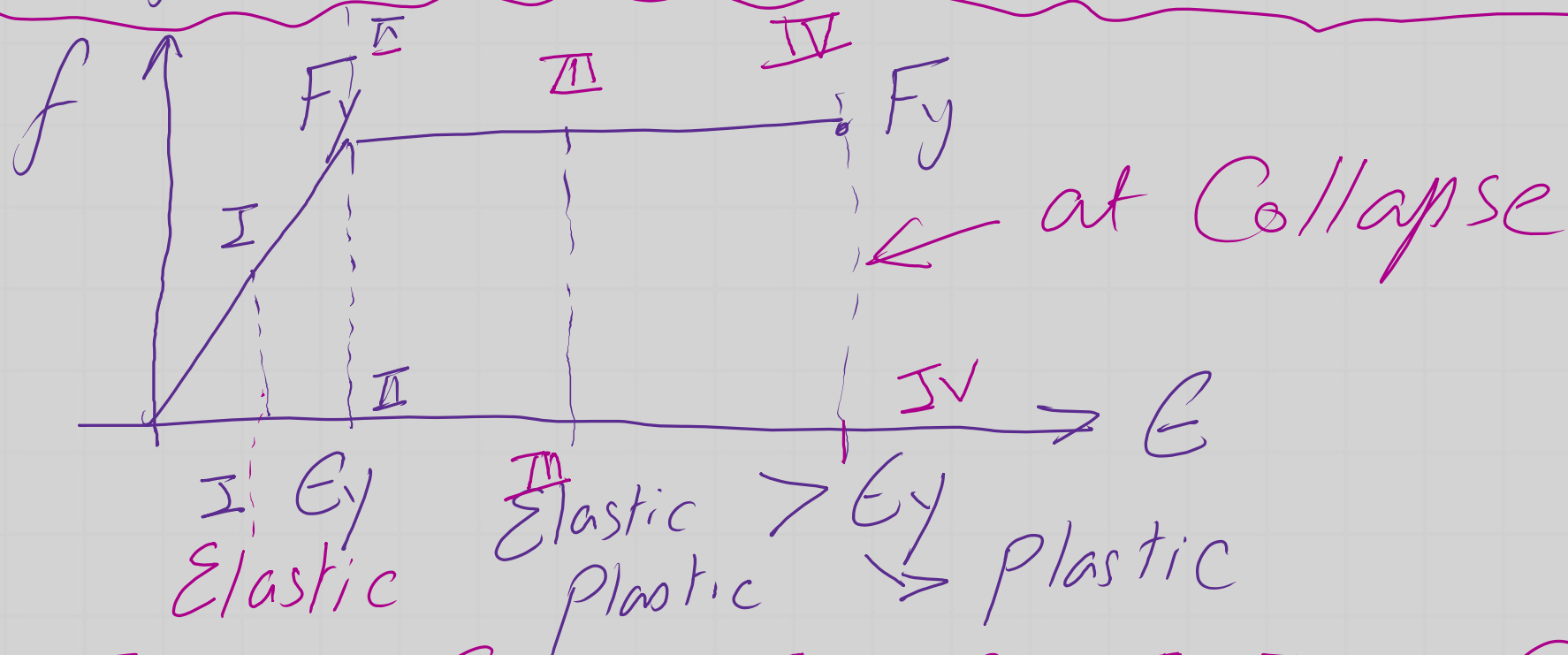
Estimate collapse load For shown beam

$$P \rightarrow P_y \Rightarrow P_p$$

What is the Plastic Hinge Length?



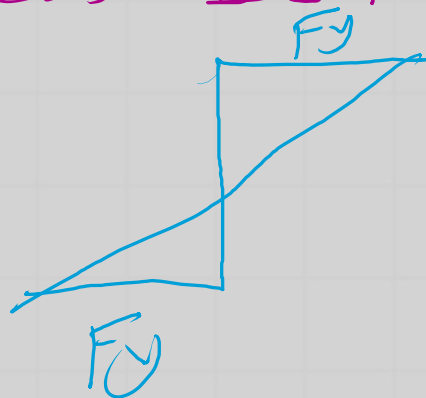
We refer to strain Versus stress Graph



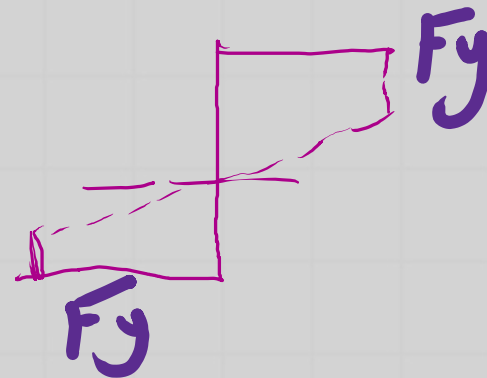
Case I - I



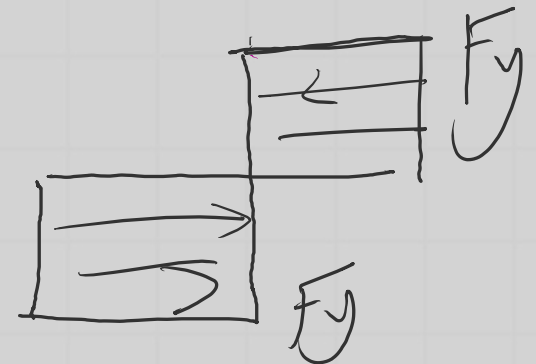
Case II - II



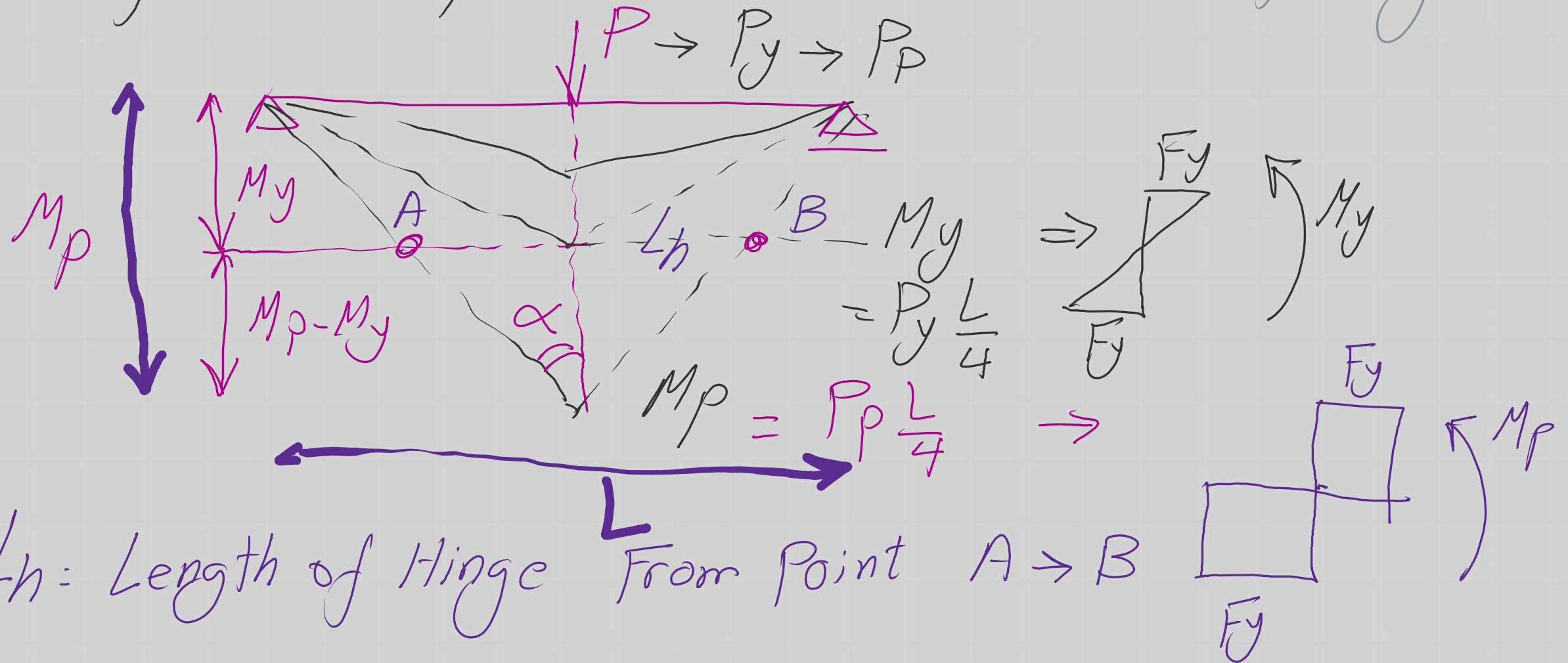
Case III - III

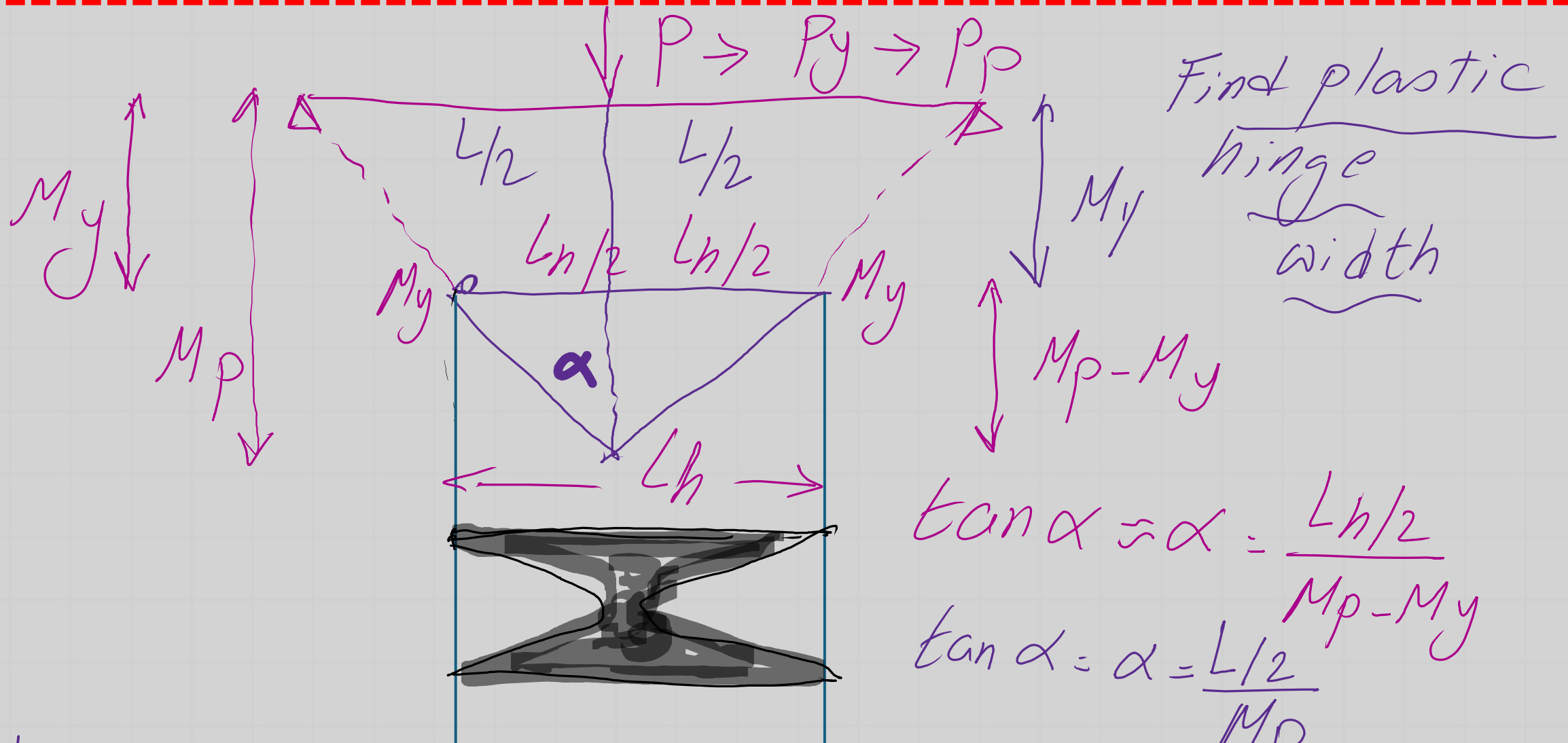


Case IV - IV



Let us Examine the Bending moment diagram of the simple beam Under CL of length L





$$\frac{L_h}{2} = (M_p - M_y) \alpha$$

$$\frac{L_h}{2} = \frac{L}{2} \frac{M_p - M_y}{M_p} = \frac{L}{2} \left(1 - \frac{M_y}{M_p} \right)$$

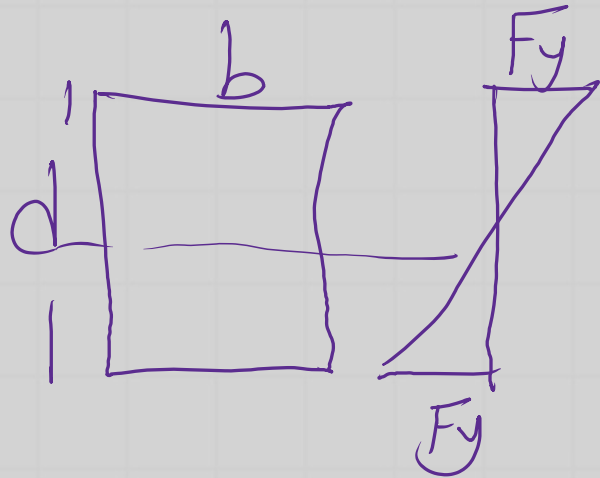
$$M_p = P_p \frac{L}{4} \quad (A)$$

$$\begin{aligned} M_p &= F_y Z_x \\ M_y &= F_y S_x \end{aligned}$$

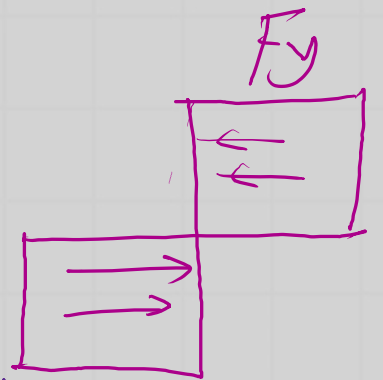
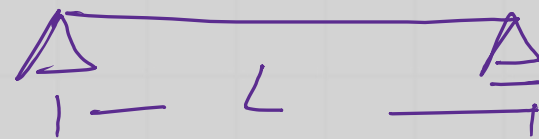
From α Equation $\frac{L_h}{2} = \frac{L}{2} \left(1 - \frac{M_y}{M_p} \right)$

Length of plastic hinge = $L_h = L \left(1 - \frac{M_y}{M_p} \right)$

For a rectangle section



$$M_y = F_y \frac{bd^2}{6}, \quad M_p = F_y \frac{bd^2}{4}$$



$$L_h = L \left(1 - \frac{bd^2}{6} \left(\frac{4}{bd^2} \right) \right) = \frac{L}{3} F_y$$

What is hinge Length for a W-section?

W18x76

$F_y = 50 \text{ ksi}$ A992 steel

Area = 22.30
inch²

$S_x = 146 \text{ inch}^3$
 $Z_x = 163 \text{ inch}^3$ } $\frac{S_x}{Z_x} = 0.8957$

Length of plastic hinge = $L_h = L \left(1 - \frac{M_y}{M_p}\right)$

$M_y = F_y S_x$

$M_p = F_y Z_x$

$L_h = L \left(1 - \frac{F_y S_x}{F_y Z_x}\right)$

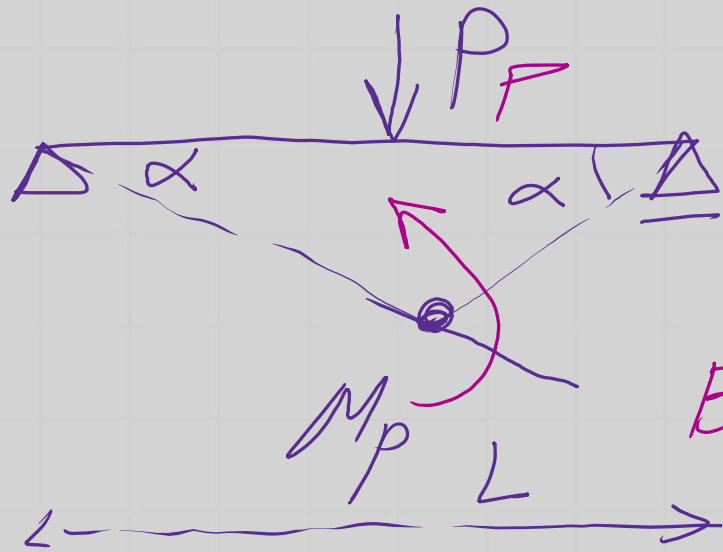
L : span length $L \left(1 - \frac{0.8957}{1}\right) = 0.1042 L$

There are Two methods to Find the relation between P_p & M_p

① Virtual Work - upper bound At Collapse Load: $\sum W_e = \sum W_i$

External Energy

internal energy



$$P_p \Delta = M_p (2\alpha)$$

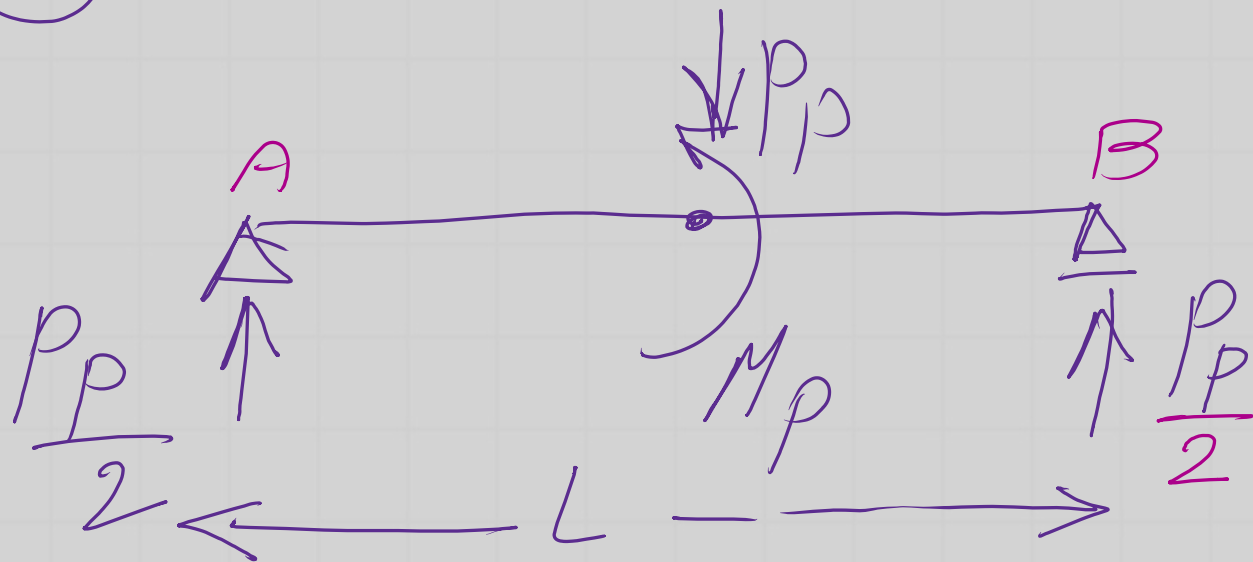
But $\alpha = \tan \alpha = \frac{\Delta}{L/2} = \frac{2\Delta}{L}$

$$P_p = \frac{M_p}{\Delta} (2) \left(\frac{2\Delta}{L} \right) = 4 \frac{M_p}{L}$$

$$\text{or } M_p = \frac{L}{4} P_p$$

② Lower bound

2nd method
at hinge we have $M = M_p$



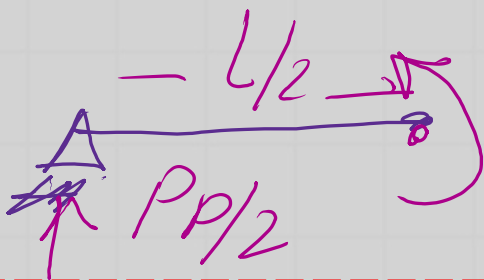
$$M_p - \frac{P_p}{2} \left(\frac{L}{2} \right) = 0$$

$$M_p = \frac{P_p L}{4}$$

Or $P_p = 4 M_p$ which is the same

result as obtained from virtual

work method



$$M(x) = \frac{P_p}{2} \frac{L}{2} = M_p \Rightarrow$$

McCormac

Fifth edition

8-21 to 8-39. Using the given sections, all of A992 steel, and the plastic theory, determine the values of P_n and w_n as indicated.

8-21. (Ans 90.68 k)

Modified $AB = BC$

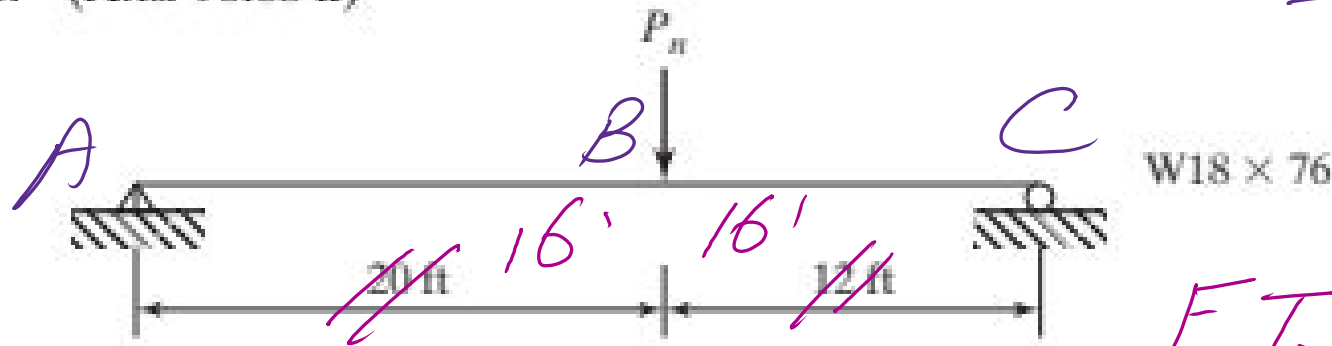


FIGURE P8-21

FT. kips = 12 inch
kips

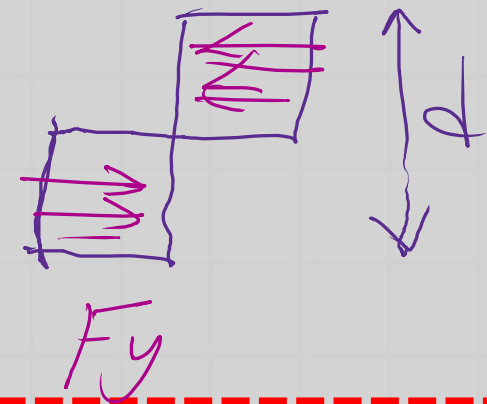
solution

A992 steel $\Rightarrow f_y = 50$ k; F_y

W18x76 $\Rightarrow I_x = 163$ inch⁴

Area = 22.30 inch²

$$M_p = F_y Z_x = 50(163) = 8150 \text{ inch-kips} / 12$$

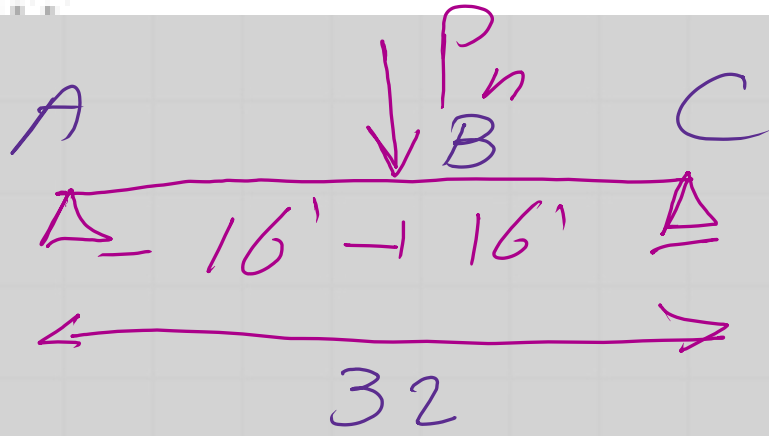


8.21

McCormac

Using the given sections, all of A992 steel, and the plastic theory, determine the values of P_n and w_n as indicated.

① Use



W18 x 76

Lower
bound

$$A = 22.30 \text{ inch}^2$$

$$Z_x = 163 \text{ inch}^3$$

$$F_y = 50 \text{ ksi}$$

$$A_y = P_n \left(\frac{16}{32} \right) = \frac{P_n}{2}$$

$$M_{\max} = P_n \cdot \left(\frac{L}{2} \right) = \frac{P_n}{2} (16) = 8 P_n \rightarrow \text{FT. kip}$$

at B

Equate $M_{\max}$ to $M_p = (8150/12) \text{ FT. kips}$

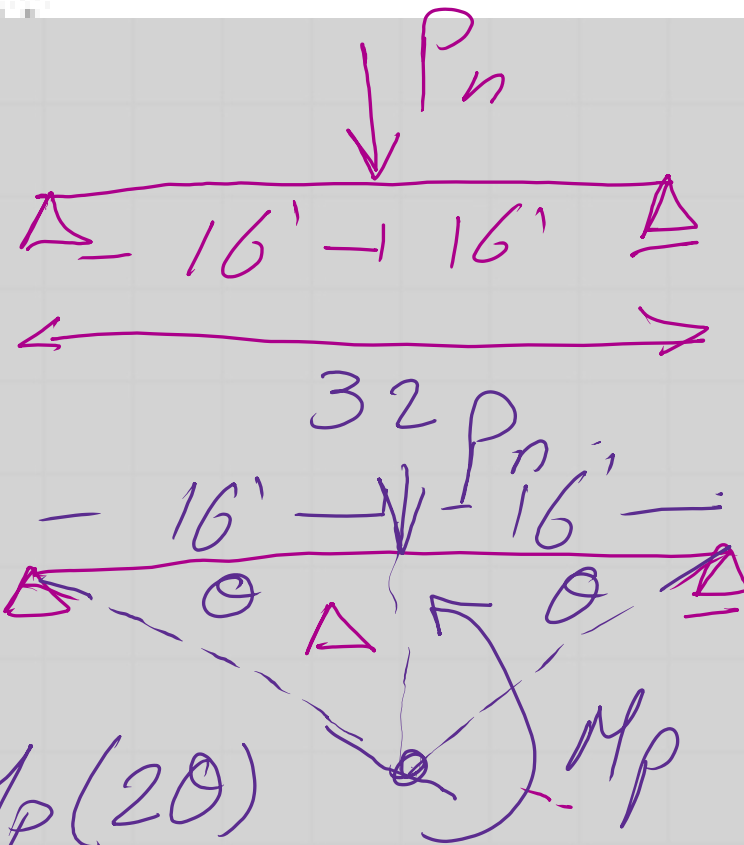
$$\rightarrow P_n = 84.90 \text{ kips}$$

8.21

McCormac

Using the given sections, all of A992 steel, and the plastic theory, determine the values of P_n and w_n as indicated.

Upper bound -



Using Virtual Work

$$P_n \cdot \Delta = M_p (2\theta)$$

$$P_n \Delta = 679.16 \left(\frac{2\Delta}{16} \right) \Rightarrow P_n = 48.896 \text{ kips} \approx 48.9 \text{ k}$$

W18 x 76

$$A = 22.30 \text{ inch}^2$$

$$Z_x = 163 \text{ inch}^3$$

$$F_y = 50 \text{ ksi}$$

$$M_p = F_y Z_x$$

$$M_p = M_n = 50 \left(\frac{163}{12} \right) = 679.16 \text{ FT. kips}$$

$$\theta = \frac{\Delta}{16}$$

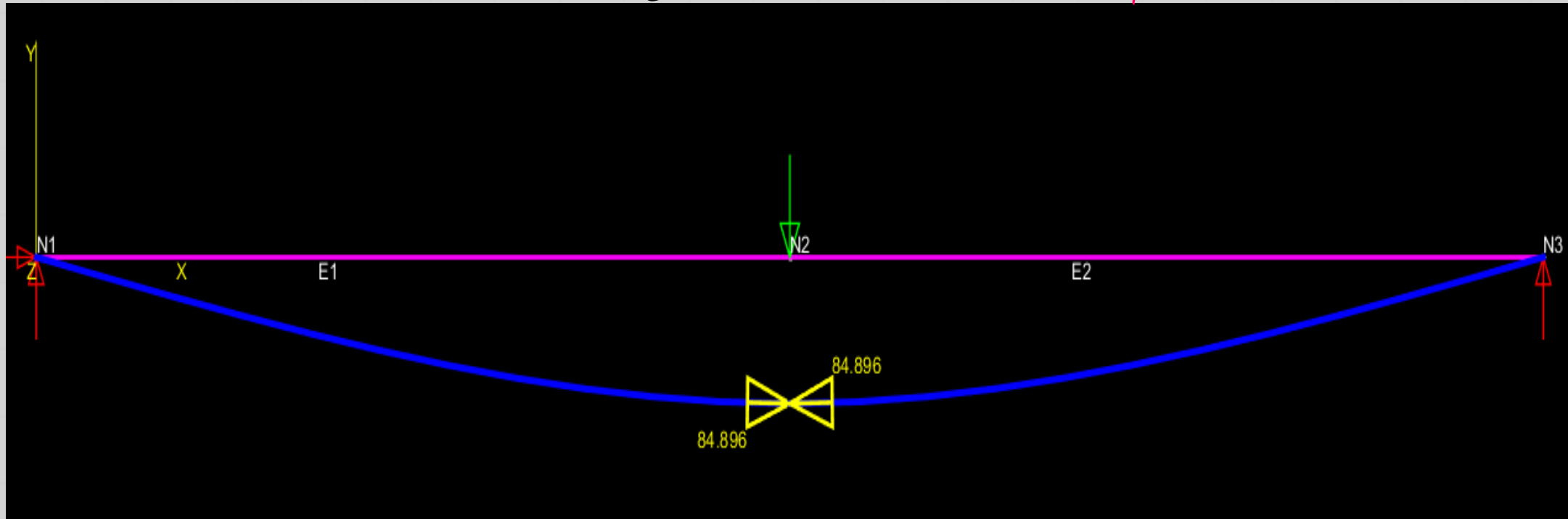
Using MASTAN 2

↓^P starts as equal 1 kips

increment size 10, Max no. of increment = 12

Max ratio = 120

W18x76



increment #9 (80 → 90)

→ Load = 84.896 kips

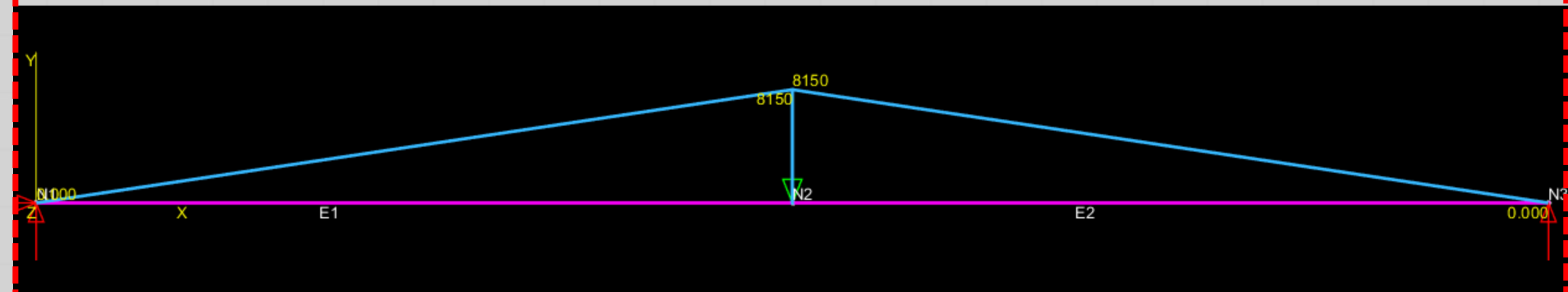
Using MASTAN 2

↓^P starts as equal 1 kips

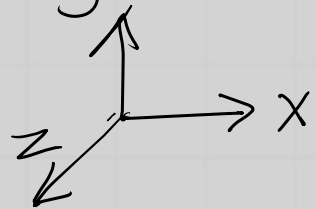
increment size 10, Max no. of increment = 12

Max ratio = 120

W12 x 76



Moment $M_z = 8150 \text{ kip} \cdot \text{inch}$



$F_y = 50 \text{ ksi}$
 $Z_x = 163 \text{ inch}^3$

$\Rightarrow M_p = 50(163) = 8150 \text{ kips} \cdot \text{inch}$

shear value V_y $R_A = 42.45 \text{ Kips} +ve = \frac{P_n}{2}$

