

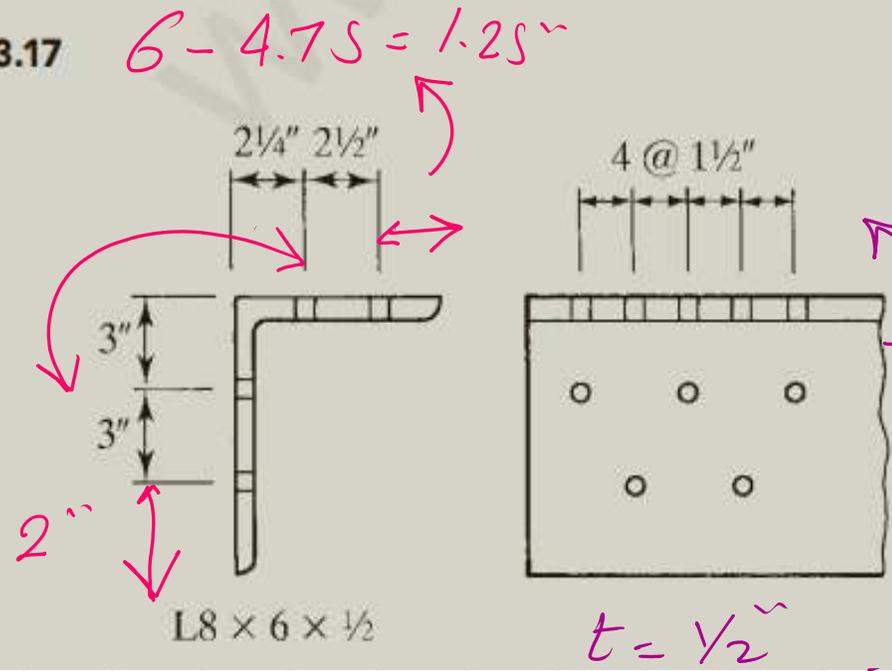
**Example 3.7**

An angle with staggered fasteners in each leg is shown in Figure 3.17. A36 steel is used, and holes are for  $\frac{7}{8}$ -inch-diameter bolts.

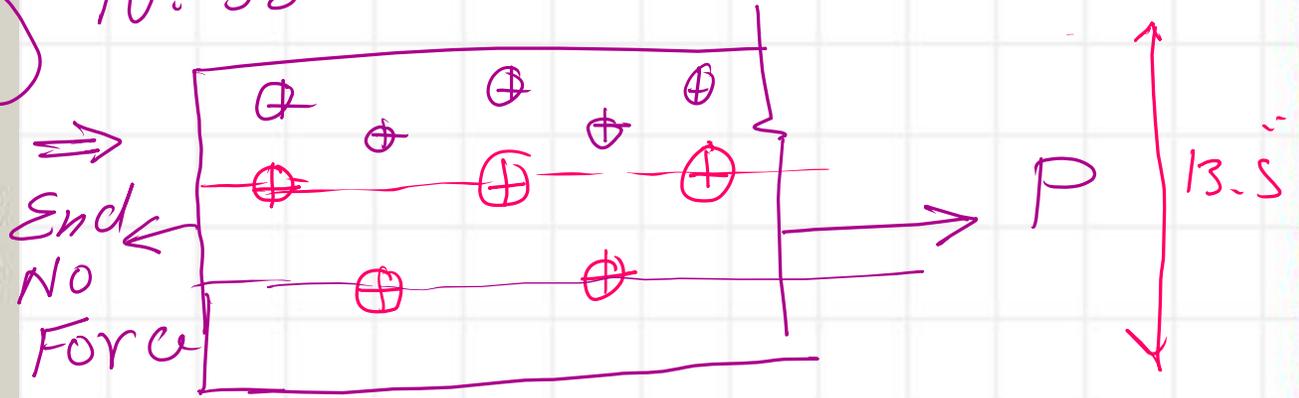
a. Determine the design strength for LRFD.

b. Determine the allowable strength for ASD.

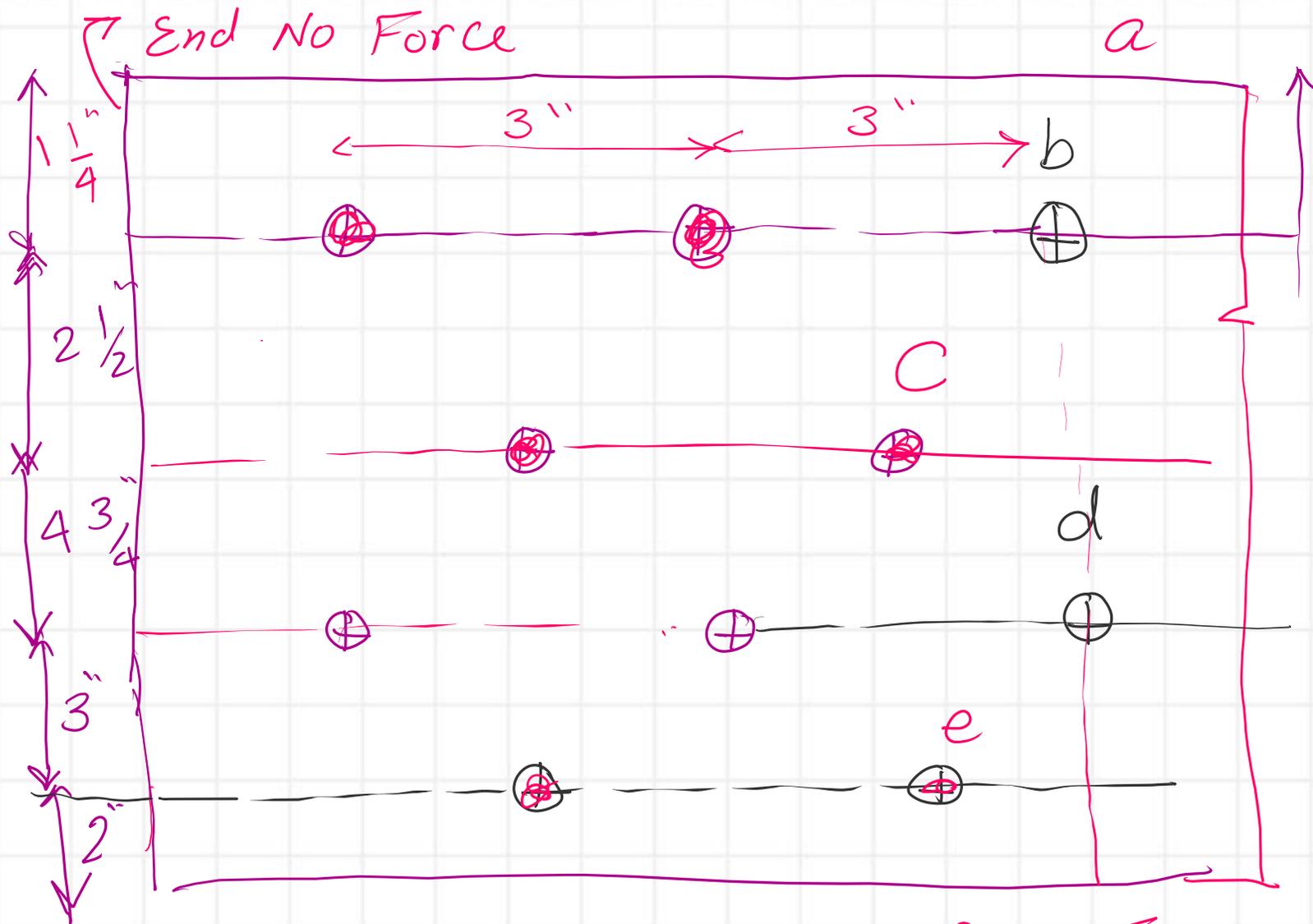
FIGURE 3.17



A36 steel  $d_b = \frac{7}{8}$   
 $F_y = 36$  ksi  
 $F_u = 58$  ksi  
 $d_h = 1$



CL From bolt  $\rightarrow$  bolt  $= 2.25 + 3 - \frac{1}{2} = 5.25 - 0.5 = 4.75$   
 Height of plate  $= 8 + 6 - \frac{t}{2} = 13.50$



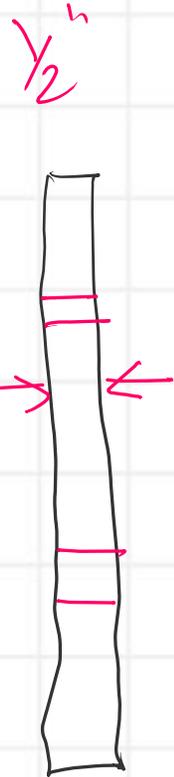
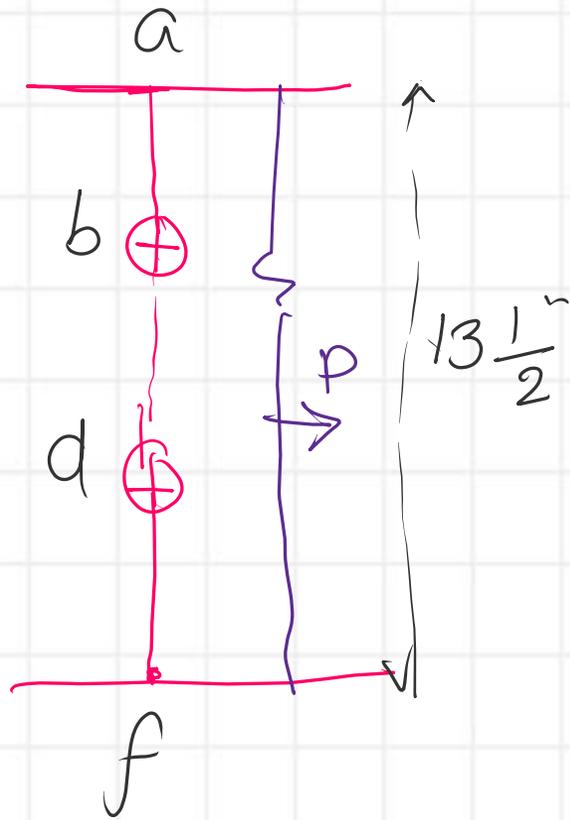
$$h = 13 \frac{1}{2} = 3 + 2 + 4 + 3 \frac{3}{4} + 2 \frac{1}{2} + 1 \frac{1}{4}$$

$$= 13 \frac{1}{2}$$

No. of bolts = 10 Each bolt carries  $\frac{P}{10}$

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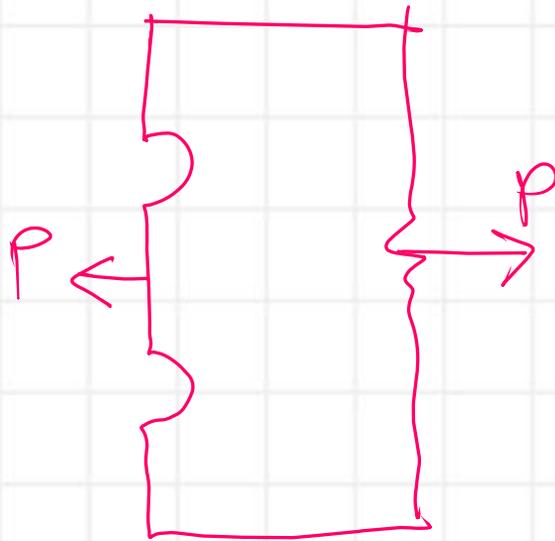
Route a b c f



$$t_p = \frac{1}{2}''$$
$$d_h = 1''$$

From Table 1-7  
 $8 \times 6 \times \frac{1}{2}'' \Rightarrow A_g =$   
 $A_g = 6.80 \text{ inch}^2$

Deduct 3 holes areas  
 $A_h = 2(1'')(\frac{1}{2}) = 1.00 \text{ inch}^2$   
 $A_n = 6.80 - 1.00 = 5.80 \text{ inch}^2$   
section  $\Rightarrow 100\% P$



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Route ABCdf

$S \rightarrow$

$d_h = 1''$ ,  $t_L = \frac{1}{2}''$

No bolts ( $A_g = 6.80 \text{ inch}^2$ )

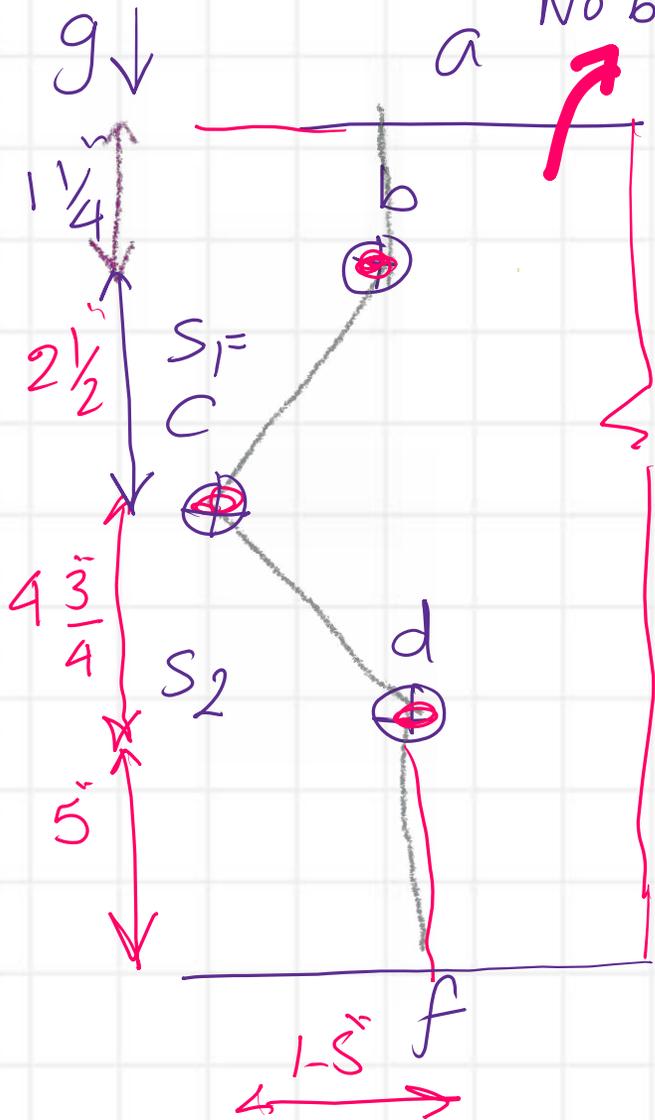
deduct 3 holes areas =  $3(1'')(\frac{1}{2}) = 1.50 \text{ inch}^2$

Add  $\sum \frac{S^2}{4g} \cdot t$

$$\rightarrow \left( \frac{1.5}{4(2.5)} \right)^2 \left( \frac{1}{2} \right) + \left( \frac{1.5}{4(7.5)} \right)^2 \left( \frac{1}{2} \right) = \frac{9}{80} + 0.237 = 0.349 \text{ inch}^2$$

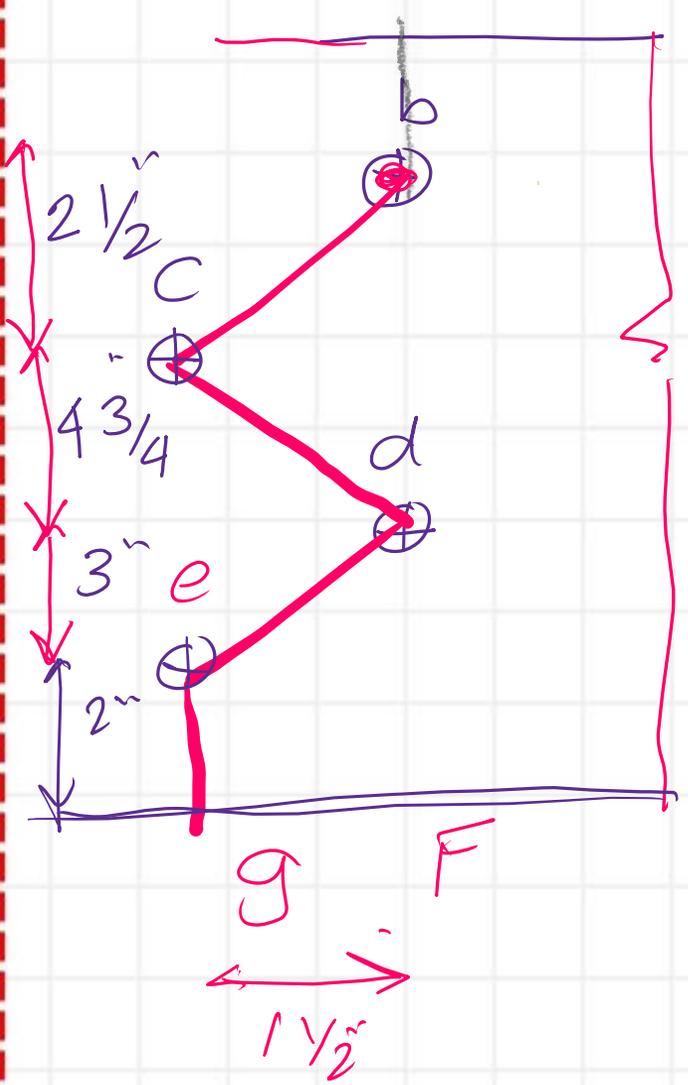
$$A_n = 6.80 - 1.50 + 0.349 = 5.649 \text{ inch}^2$$

$\Rightarrow 100\% P$



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Route a b c d e g



$$A_g = 6.80 \text{ inch}^2$$

$$d_h = 1'' , t_L = \frac{1}{2}''$$

deduct holes areas =  $4 (1'') (\frac{1}{2}) = 2.00 \text{ inch}^2$

$$\text{Add } \sum \frac{S^2}{4g} \cdot t$$

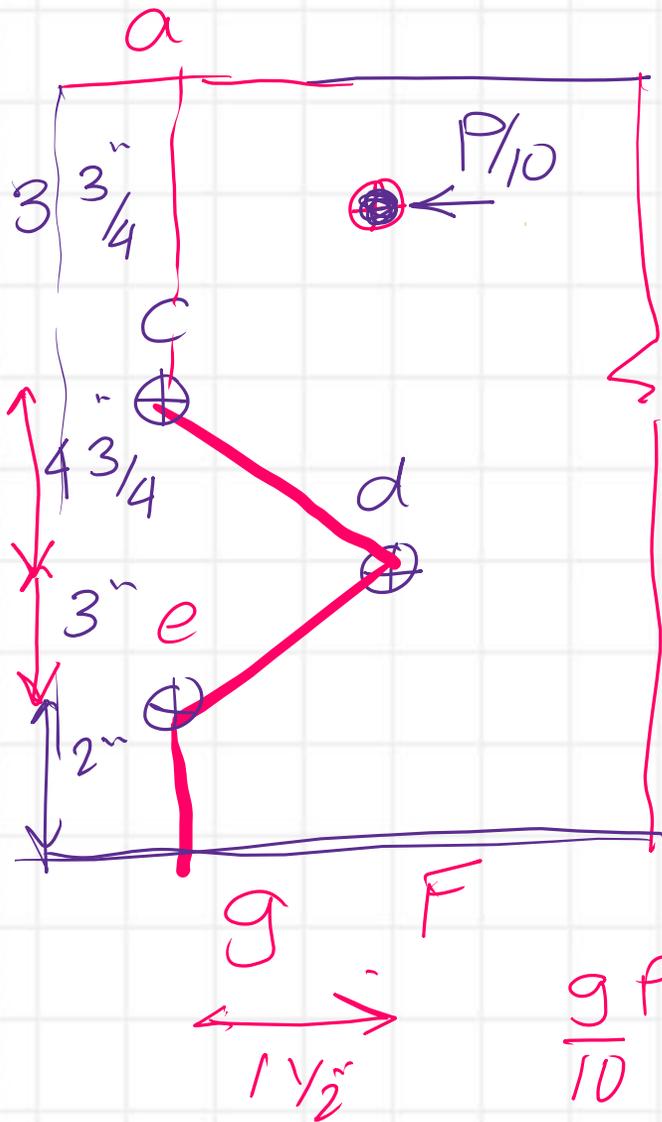
$$\rightarrow \left( \frac{1.5^2}{4(2.5)} \right) \left( \frac{1}{2} \right) + \left( \frac{1.5^2}{4(4.75)} \right) \left( \frac{1}{2} \right) + \left( \frac{1.5^2}{4(3)} \right) \left( \frac{1}{2} \right)$$

$$= \frac{9}{80} + \frac{9}{152} + \frac{3}{32} = 0.265 \text{ inch}^2$$

$$A_n = 6.80 - 2.0 + 0.265 = 5.065 \text{ inch}^2$$

$\Rightarrow 100\% P$

Route acdeg



$$A_g = 6.80 \text{ inch}^2$$

$$d_h = 1'' , t_L = \frac{1}{2}''$$

deduct holes areas =  $3(1'')(\frac{1}{2}) = 1.50 \text{ inch}^2$

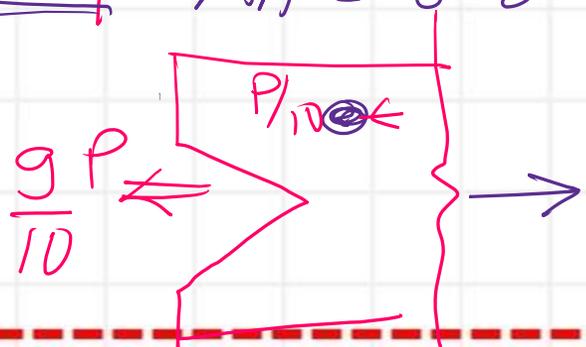
$$\text{Add } \sum \frac{S^2}{4g} \cdot t$$

$$\left( \frac{(1.5)^2}{4(4.75)} \right) \left( \frac{1}{2} \right) + \left( \frac{(1.5)^2}{4(3)} \right) \left( \frac{1}{2} \right)$$

$$= \frac{9}{152} + \frac{3}{32} = 0.153 \text{ inch}^2$$

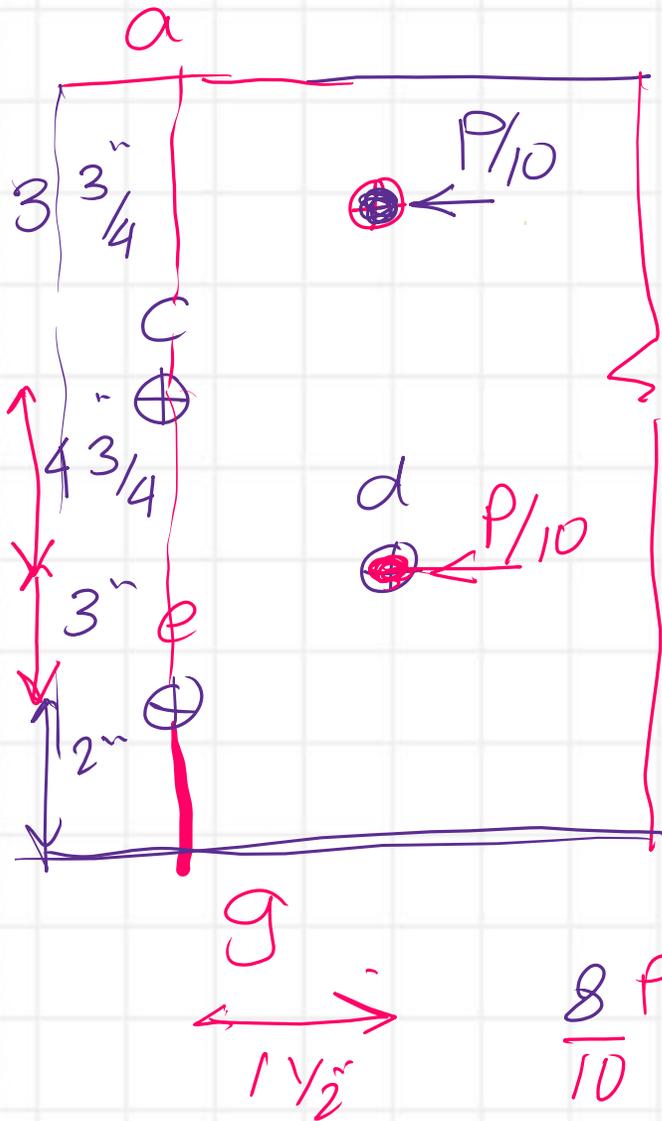
$$A_n = 6.80 - 1.5 + 0.153 = 5.453 \text{ inch}^2$$

$$\Rightarrow \frac{9}{10} P \quad A_n = 5.453 \left( \frac{10}{9} \right) = 6.059 \text{ inch}^2$$



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Route aceg



$$A_g = 6.80 \text{ inch}^2$$

$$d_h = 1'' , t_L = \frac{1}{2}''$$

deduct holes areas =  $2(1'')(\frac{1}{2}) = 1.0 \text{ inch}^2$

No staggered bolts

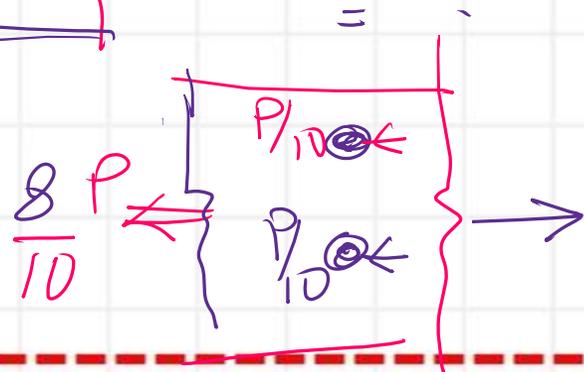
$$A_n = 6.80 - 1.0 = 5.80 \text{ inch}^2$$

⇒ section has  $\frac{8}{10} P$

$$A_n = \frac{10}{8}(5.80) = 7.25 \text{ inch}^2$$

⇒  $\frac{8}{10} P$

$$A_n = \frac{10}{8}(7.25) = 9.06 \text{ inch}^2$$



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# Summary of Routes

abcf

abcdf

abcdeg

acdeg

aceg

$$\underline{A_n = 100 \text{ k}}$$

$$5.80 \text{ inch}^2$$

$$5.649 \text{ inch}^2$$

$$5.065 \text{ inch}^2 \Rightarrow \text{min}$$

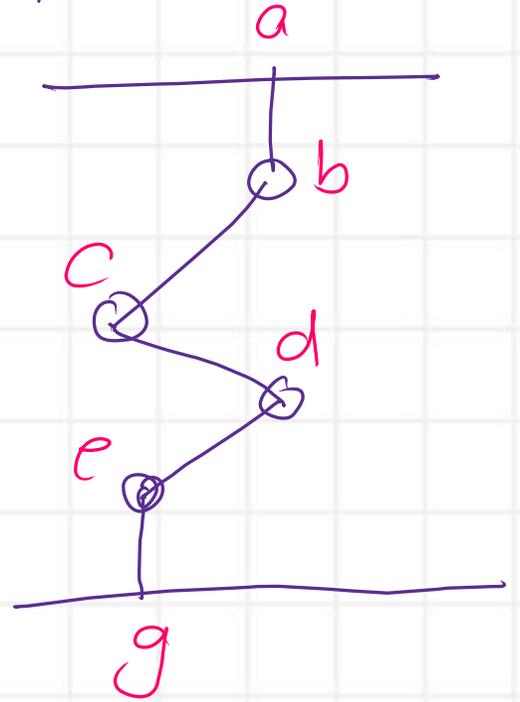
$$6.059 \text{ inch}^2$$

$$9.06 \text{ inch}^2$$

$$A_g = 6.80 \text{ inch}^2$$

$$F_y = 36 \text{ ksi}$$

$$F_u = 58 \text{ ksi}$$



Select min  $A_n = 5.065 \text{ inch}^2$   $U = 1$   $A_e = 5.065 \text{ inch}^2$

LRFD Tensile yielding  $\phi A_g F_y = 0.9(6.8)(36)$

$$= 220.32 \text{ k} \Rightarrow 220 \text{ kips}$$

Tensile rupture

$$\phi A_e \cdot F_u = 0.75(5.065)(58) = 220.33 \text{ k}$$

# Summary of Routes

abcf

abcdf

abcdeg

acdeg

aceg

$$A_n = 100 \text{ in}^2$$

$$5.80 \text{ inch}^2$$

$$5.649 \text{ inch}^2$$

$$5.065 \text{ inch}^2 \Rightarrow \text{min}$$

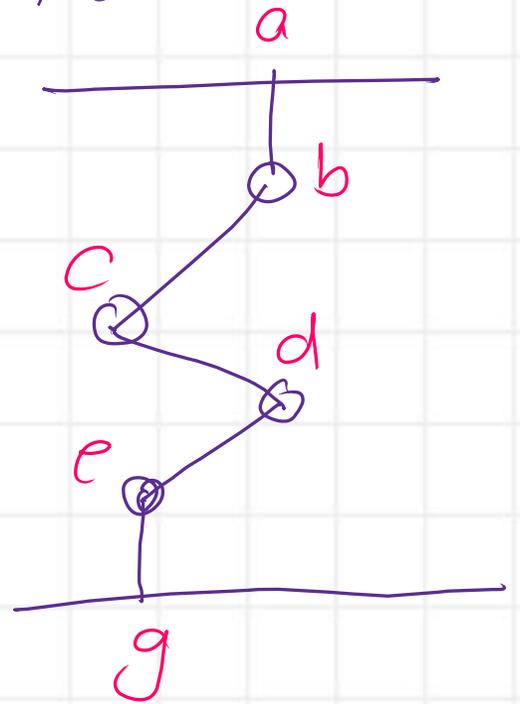
$$6.059 \text{ inch}^2$$

$$9.06 \text{ inch}^2$$

$$A_g = 6.80 \text{ inch}^2$$

$$F_y = 36 \text{ ksi}$$

$$F_u = 58 \text{ ksi}$$



Select min  $A_n = 5.065 \text{ inch}^2$   $U = 1$   $A_e = 5.065 \text{ inch}^2$

ASD Tensile Yielding  $\frac{1}{\phi} A_g F_y = \frac{1}{1.67} (6.8)(36) = 146.59 \text{ kips} \Rightarrow 147 \text{ kips}$

Tensile rupture

$$\frac{1}{\phi} A_e \cdot F_u = \frac{1}{2} (5.065)(58) = 146.89 \text{ kips}$$