

**Example 10.1.** Bolts in Shear and Bearing with Deformation a Design Consideration

The connection shown in Fig. 10.7 consists of four, grade A490,  $\frac{3}{4}$ -in-diameter bolts. The bolts are snug-tight and threads are excluded from the shear planes. Deformation around the bolt holes is a design consideration and the bolt spacing is as indicated. The angles and gusset plate are fabricated from A36 steel. Assuming that the angles and gusset plate are satisfactory, determine the shear force that may be applied to the bolts in the connection.

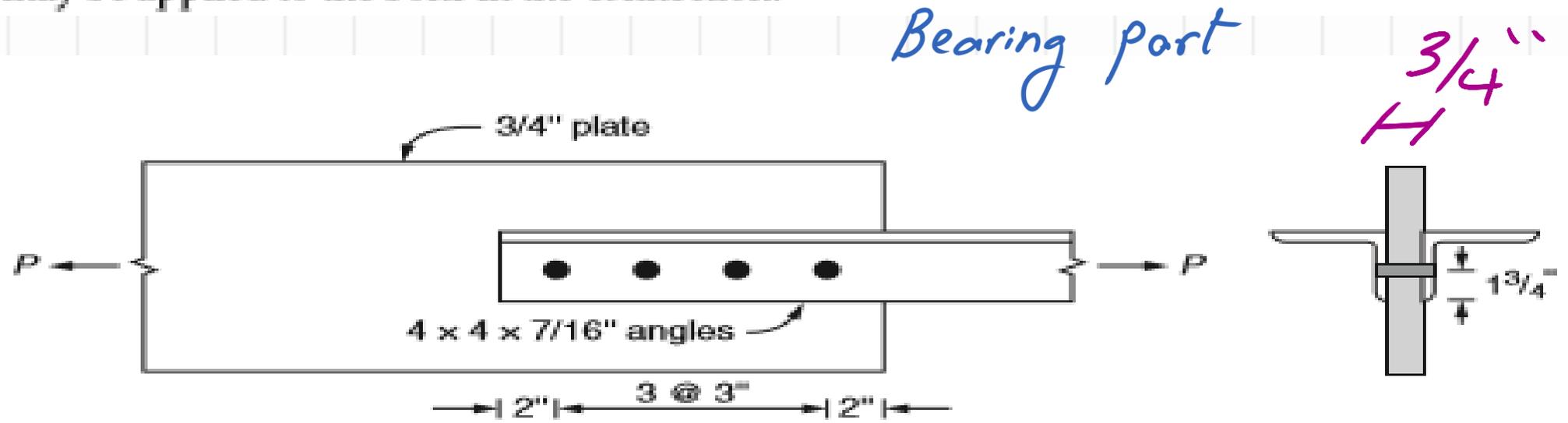


FIGURE 10.7 Details for Example 10.1.

$d_b = \frac{3}{4}$ " X-Type  
 $S = 3"$

$N_o = 4$

Prepared by Eng. Maged Kamel.

Plate A36  
 $F_u = 58 \text{ ksi}$

### Example 10.1. Bolts in Shear and Bearing with Deformation a Design Consideration

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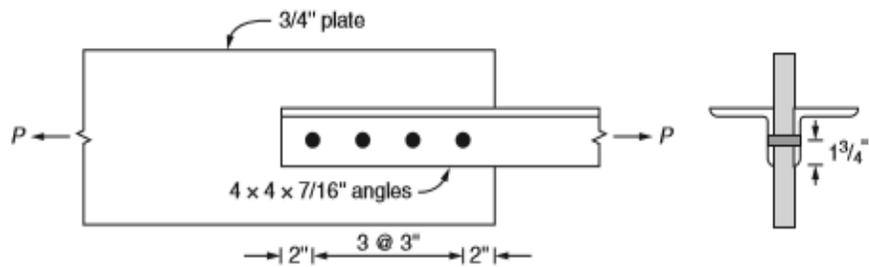
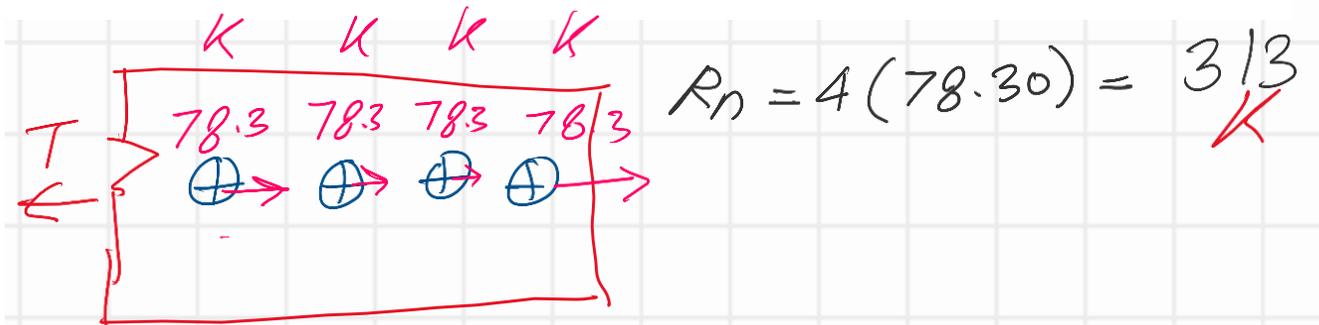
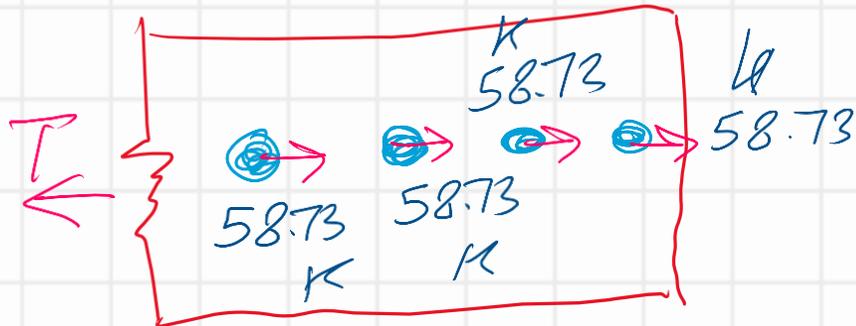


FIGURE 10.7 Details for Example 10.1.

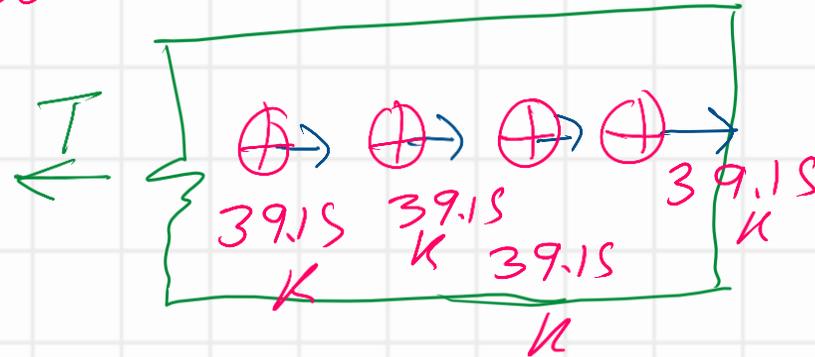


LRFD  $\phi = 0.75 \times 78.30 = 58.73 \text{ K}$



ASD  $\Omega = 2$

$$\frac{1}{2}(78.30) = 39.15 \text{ K}$$



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# Check ASD Value For Bearing For inner bolts

**Table 7-4 Available Bearing Strength at Bolt Holes Based on Bolt Spacing**  
kips/in. thickness

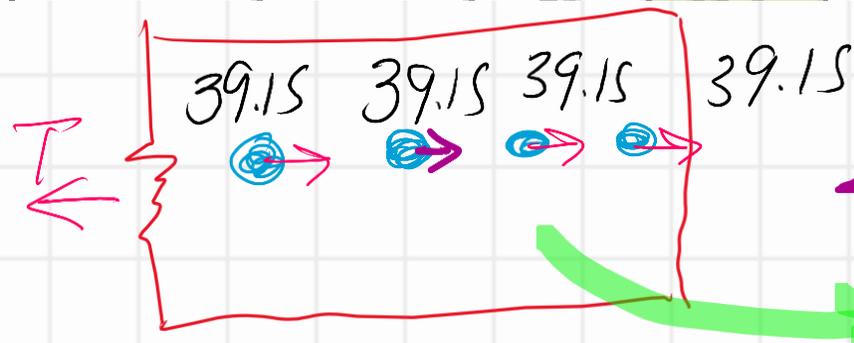
$d_b = 3/4"$

inner bolts

$F_u = 58$   
ksi

Hole Type	Bolt Spacing, S, in.	$F_u$ , ksi	Nominal Bolt Diameter, d, in.							
			$2\frac{3}{4}$		$3\frac{1}{4}$		$3\frac{3}{4}$		4	
			$t_p/d$	$\phi$	$t_p/d$	$\phi$	$t_p/d$	$\phi$	$t_p/d$	$\phi$
			ASD	LRFD	ASD	LRFD	ASD	LRFD	ASD	LRFD
STD SSLT	$2\frac{3}{4} d_b$	58 65	34.1 38.2	51.1 57.3	41.3 46.3	62.0 69.5	48.6 54.4	72.9 81.7	55.8 62.6	83.7 93.8
	3 in.	58 65	43.5 48.8	65.3 73.1	52.2 58.5	78.3 87.8	60.9 68.3	91.4 102	67.4 75.6	101 113
SSLP	$2\frac{3}{4} d_b$	58 65	27.6 30.9	41.3 46.3	34.8 39.0	52.2 58.5	42.1 47.1	63.1 70.7	47.1 52.8	70.7 79.2
	3 in.	58 65	43.5 48.8	65.3 73.1	52.2 58.5	78.3 87.8	60.9 68.3	91.4 102	58.7 65.8	88.1 98.7
OVS	$2\frac{3}{4} d_b$	58 65	29.7 33.3	44.6 50.0	37.0 41.4	55.5 62.2	44.2 49.6	66.3 74.3	49.3 55.3	74.0 82.9
	3 in.	58 65	43.5 48.8	65.3 73.1	52.2 58.5	78.3 87.8	60.9 68.3	91.4 102	60.9 68.3	91.4 102
LSLP	$2\frac{3}{4} d_b$	58 65	3.62 4.06	5.44 6.09	4.35 4.88	6.53 7.31	5.08 5.69	7.61 8.53	5.80 6.50	8.70 9.75
	3 in.	58 65	43.5 48.8	65.3 73.1	39.2 43.9	58.7 65.8	28.3 31.7	42.4 47.5	17.4 19.5	26.1 29.3

3"  
→



From calculations

$P_L > t_{PL} = 3/4"$ ,  $S = 3"$

$\frac{1}{t_b} r_n = 52.2 \frac{k}{inch} (\frac{3}{4}) = 39.15 \text{ kips}$   
Same value for 3 bolts

$L_e = 2''$   $F_U = 58 \text{ ksi}$   $t_{PL} = 3/4''$  A36

Table 7-5

**Available Bearing Strength at Bolt Holes Based on Edge Distance**

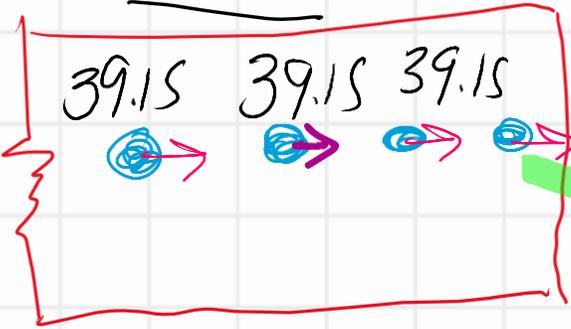
$d_b = 3/4''$

kips/in. thickness

Hole Type	Edge Distance $L_e$ , in.	$F_u$ , ksi	Nominal Bolt Diameter, $d$ , in.							
			$5/8$		$3/4$		$7/8$		1	
			$r_n/\Omega$	$\phi r_n$	$r_n/\Omega$	$\phi r_n$	$r_n/\Omega$	$\phi r_n$	$r_n/\Omega$	$\phi r_n$
			ASD	LRFD	ASD	LRFD	ASD	LRFD	ASD	LRFD
STD SSLT	1 1/4	58 65	31.5 35.3	47.3 53.0	29.4 32.9	44.0 49.4	27.2 30.5	40.8 45.7	25.0 28.0	37.5 42.0
	2	58 65	43.5 48.8	65.3 73.1	52.2 58.5	78.3 87.8	53.3 59.7	79.9 89.6	51.1 57.3	76.7 85.9
SSLP	1 1/4	58 65	28.3 31.7	42.4 47.5	26.1 29.3	39.2 43.9	23.9 26.8	35.9 40.2	20.7 23.2	31.0 34.7
	2	58 65	43.5 48.8	65.3 73.1	52.2 58.5	78.3 87.8	50.0 56.1	75.0 84.1	46.8 52.4	70.1 78.6
OVS	1 1/4	58 65	29.4 32.9	44.0 49.4	27.2 30.5	40.8 45.7	25.0 28.0	37.5 42.0	21.8 24.4	32.6 36.6
	2	58 65	43.5 48.8	65.3 73.1	52.2 58.5	78.3 87.8	51.1 57.3	76.7 85.9	47.9 53.6	71.8 80.4
ICSP	1 1/4	58	16.3	24.5	10.9	16.3	5.44	8.16	—	—
		65	18.3	27.4	12.2	18.3	6.09	9.14	—	—

ASD value

$\frac{r_n}{\Omega} = \frac{52.2 \text{ kips}}{\text{inch}}$   $t_o = 3/4''$



39.15 k

$\frac{r_n}{\Omega} = 52.2 \left(\frac{3}{4}\right) = 39.15 \text{ kips}$

Same For Exterior bolt



Hence, tear out does not govern and a nominal bearing stress of  $2.4F_u$  is applicable. The available bearing capacity of the four bolts on the 3/4-in plate material is

LRFD	ASD
<p>The design bearing strength is</p> $\phi R_n = \phi n \times 2.4dtF_u$ $= 0.75 \times 4 \times 2.4 \times 0.75 \times 0.75 \times 58$ $= 235 \text{ kips}$	<p>The allowable bearing strength is</p> $R_n / \Omega = n \times 2.4dtF_u / \Omega$ $= 4 \times 2.4 \times 0.75 \times 0.75 \times 58 / 2$ $= 157 \text{ kips}$

*The author  
Summary*

The double shear capacity of the four, grade A490, 3/4-in-diameter bolts with threads excluded from the shear planes is

LRFD	ASD
<p>The design shear strength is</p> $\phi_v R_{nv} = 2n\phi_v F_{nv} A_b$ $= 2 \times 4 \times 0.75 \times 84 \times 0.442$ <p> = 223 kips ... governs</p> $< 235 \text{ kips}$	<p>The allowable shear strength is</p> $R_{nv} / \Omega_v = 2nF_{nv} A_b / \Omega_v$ $= 2 \times 4 \times 84 \times 0.442 / 2$ <p> = 149 kips ... governs</p> $< 157 \text{ kips}$

**Prepared by Eng. Maged Kamel.**