

A simply supported beam of grade 50 steel is laterally braced at 4 ft intervals. If the beam is subjected to a uniform factored bending moment of 270 ft-kips (LRFD) or 180 ft-kips (ASD), with $C_b = 1.0$, determine (a) the lightest adequate W shape, and (b) the W shape with the minimum allowable depth.

Alan Williams - Structural
Engineering Reference Manual-
Professional Publications

Example

4-5

Design

A simply supported beam of grade 50 steel is laterally braced at 4 ft intervals. If the beam is subjected to a uniform factored bending moment of 270 ft-kips (LRFD) or 180 ft-kips (ASD), with $C_b = 1.0$, determine (a) the lightest adequate W shape, and (b) the W shape with the minimum allowable depth.

ALAN
Williams

8th
rev.

Solution

Grade 50

ASD

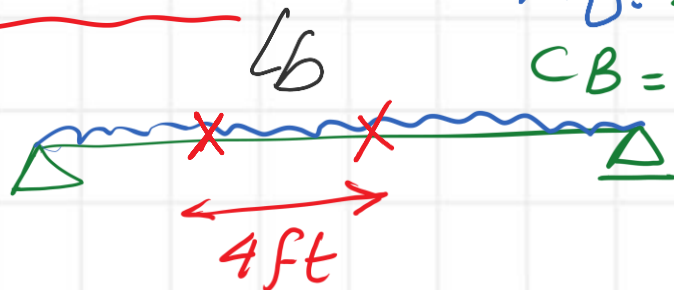
$$M_U = 270 \text{ FT-kips}$$

$$\leftrightarrow 180 \text{ FT-kips}$$

$$C_b = 1$$

$$f_y = 50 \text{ ksi}$$

$L_b = 4'$
distance
between
bracings



$$\text{For } M_n \phi \geq M_U \rightarrow 0.90 f_y Z_x \geq 270$$

$$Z_x = \frac{270}{0.90(50)} \times 12 = 72 \text{ inch}^3 \text{ check table}$$

Prepared by Eng. Maged Kamel.

Given M_n Use $M_{ult} \leq \phi_b M_n$

only with $F_y = 50 \text{ ksi}$
Use Table 3-2

Z_x

Use

$\phi_b = 0.90$
 $\frac{M_{ult}}{\phi_b} = M_n = F_y Z_x$
 $Z_x = \frac{M_n}{\phi F_y}$

Table 3-2

Table 1-1

From

get

r_y

select section W - with L_p & ϕM_p
 L_r & ϕM_r

Find L_p Equation

Check whether $L_b < L_p \rightarrow M_n = F_y Z_x \text{ act}$

$F_y = 50 \text{ ksi}$

Table 3-2 (continued)
W-Shapes
Selection by Z_x

Z_x

L_p L_r No L_o c

Given
Mult

Shape	Z_x in. ³	M_{px}/Ω_b	$\phi_b M_{px}$	M_{rx}/Ω_b	$\phi_b M_{rx}$	BF/Ω_b	$\phi_b BF$	L_p ft	L_r ft	I_x in. ⁴	V_{tx}/Ω_v	$\phi_v V_{tx}$
		kip-ft	kip-ft	kip-ft	kip-ft	kips	kips				kip-ft	kip-ft
		ASD	LRFD	ASD	LRFD	ASD	LRFD				ASD	LRFD
W16x40	73.0	182	274	113	170	6.67	10.0	5.55	15.9	518	97.6	146
W12x50	71.9	179	270	112	169	3.97	5.98	6.92	23.8	391	90.3	135
W8x67	70.1	175	263	105	159	1.75	2.59	7.49	47.6	272	103	154
W14x43	69.6	174	261	109	164	4.88	7.28	6.68	20.0	428	83.6	125
W10x54	66.6	166	250	105	158	2.48	3.75	9.04	33.6	303	74.7	112

270
FT. kips

↓ ↓

L_p ↓ L_r ↓

$\phi_b M_{px}$ $\phi_b M_{rx}$

* Shape exceeds compact limit for flexure with $F_y = 50 \text{ ksi}$.

ASD $\Omega_b = 1.67$
LRFD $\phi_b = 0.90$
 $\Omega_v = 1.50$ $\phi_v = 1.00$

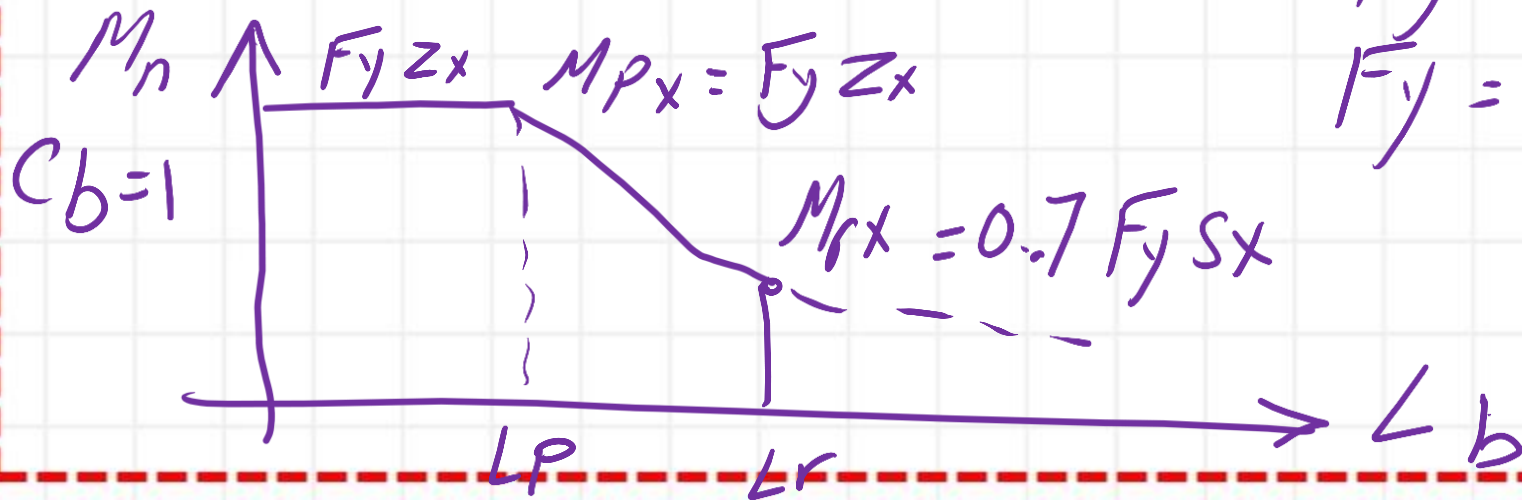
Select $Z_x \geq 72 \text{ inch}^3 \rightarrow$ Use W16x40 FT. kips
 $L_p = 5.55$ & $L_r = 15.90$ $\phi_b M_{px} = 274$
 $\phi_b M_{rx} = 170 \text{ FT. kip}$

We can use a given r_y for a section to find L_p & L_r values from Table 1-1

$$L_p = 1.76 r_y \sqrt{\frac{E}{F_y}} \Rightarrow \begin{array}{l} F_y = 36 \text{ ksi}; L_p = 49.953 r_y \\ F_y = 42 \text{ ksi}; L_p = 46.247 r_y \\ F_y = 45 \text{ ksi}; L_p = 44.679 r_y \\ F_y = 50 \text{ ksi}; L_p = 42.386 r_y \\ F_y = 65 \text{ ksi}; L_p = 37.175 r_y \end{array}$$

$$E = 29000 \text{ ksi}$$

F_y based on required value



Properties of W16x40

LRFD

Table 1-1 (continued)
W-Shapes
Properties



Nominal WT. lb/ft	Compact Section Criteria		Axis X-X				Axis Y-Y				r_{tw} in.	h_o in.	Torsional Properties	
	$\frac{b_f}{2t_f}$	$\frac{h}{t_w}$	I in. ⁴	S in. ³	r in.	Z in. ³	I in. ⁴	S in. ³	r in.	Z in. ³			J in. ⁴	C_w in. ⁶
50	5.61	37.4	659	81.0	6.68	92.0	37.2	10.5	1.59	16.3	1.89	15.7	1.52	2270
45	6.23	41.1	586	72.7	6.65	82.3	32.8	9.34	1.57	14.5	1.87	15.5	1.11	1990
→ 40	6.93	46.5	518	64.7	6.63	73.0	28.9	8.25	1.57	12.7	1.86	15.5	0.794	1730
36	8.12	48.1	448	56.5	6.51	64.0	24.5	7.00	1.52	10.8	1.83	15.5	0.545	1460

$r_y = 1.57'' \rightarrow \text{get } L_p = 42.386 (1.57) / 12 = 5.55'$
 Check $\phi_b Z_x F_y = 0.90 (73) (50) / 12 \Rightarrow 274 \text{ FT.kips}$
 $\rightarrow > \text{Mult } 270 \text{ FT.kips} \quad L_p < L_b = 4 \text{ Ft}$

Get results From Table 3-2

Table 3-2 (continued)
W-Shapes
Selection by Z_x

$F_y = 50$ ksi

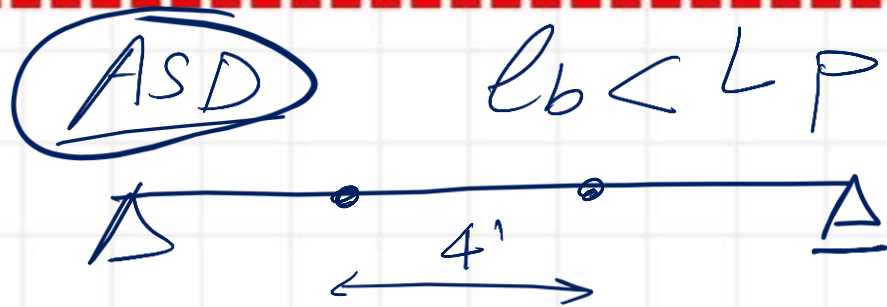
Z_x

M_{px} $L_p = 5.55'$

Shape	Z_x in. ³	M_{px}/Ω_b	$\phi_b M_{px}$	M_{rx}/Ω_b	$\phi_b M_{rx}$	BF/Ω_b	$\phi_b BF$	L_p ft	L_r ft	I_x in. ⁴	V_{max}/Ω_v	$\phi_v V_{max}$
		kip-ft	kip-ft	kip-ft	kip-ft	kips	kips				kips	kips
		ASD	LRFD	ASD	LRFD	ASD	LRFD				ASD	LRFD
W16×40	73.0	182	274	113	170	6.67	10.0	5.55	15.9	518	97.6	146
W12×50	71.9	179	270	112	169	3.97	5.98	6.92	23.8	391	90.3	135
W8×67	70.1	175	263	105	159	1.75	2.59	7.49	47.6	272	103	154
W14×43	69.6	174	261	109	164	4.88	7.28	6.68	20.0	428	83.6	125
W10×54	66.6	166	250	105	158	2.48	3.75	9.04	33.6	303	74.7	112

ASD	LRFD	† Shape exceeds compact limit for flexure with $F_y = 50$ ksi.
$\Omega_b = 1.67$ $\Omega_v = 1.50$	$\phi_b = 0.90$ $\phi_v = 1.00$	

$\phi_b M_n$ From Table 3-2 = $\phi_b M_{px} = 274$ FT. Kip^s
 $L_b = 4' < L_p < 5.55'$ } Same result as obtained From Table 1-1



Given $M_T = 180 \text{ FT. kips}$

Find Z_x

Given $M_T = 180 \text{ FT. kips}$

$F_y = 50 \text{ ksi}$
 $\Omega_b = 1.67$

From

For $\frac{M_n}{\Omega_b} = M_T \Rightarrow \frac{F_y Z_x}{\Omega} = 180 \rightarrow (12) \Rightarrow \text{Ft. kips}$
 inch^3

$Z_x = \Omega \frac{(180)}{F_y} = 1.67 \left(\frac{180}{50} \right) (12) = 72.144 \text{ inch}^3$
 $\Rightarrow \text{required}$

Use Table 3-2 to Find W section

$Z_x \text{ required} = 72.14 \text{ inch}^3$

$Z_x \text{ req} = 72.14 \text{ in}^3$

ASD Design

$F_y = 50 \text{ ksi}$

Sorted Z_x

Table 3-2 (continued)
W-Shapes
Selection by Z_x

$L_b = 4'$
given

Z_x

Shape	Z_x in. ³	M_{px}/Ω_b	$\phi_b M_{px}$	M_{rx}/Ω_b	$\phi_b M_{rx}$	BF/Ω_b	$\phi_b BF$	L_p ft	L_r ft	I_x in. ⁴	V_{nx}/Ω_v	$\phi_v V_{nx}$
		kip-ft	kip-ft	kip-ft	kip-ft	kips	kips				kips	kips
		ASD	LRFD	ASD	LRFD	ASD	LRFD				ASD	LRFD
W16x40	73.0	182	274	113	170	6.67	10.0	5.55	15.9	518	97.6	146
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W14x43	69.6	174	261	109	164	4.88	7.28	6.68	20.0	428	83.6	125
W10x54	66.6	166	250	105	158	2.48	3.75	9.04	33.6	303	74.7	112

ASD

LRFD

[†] Shape exceeds compact limit for flexure with $F_y = 50 \text{ ksi}$.

$\Omega_b = 1.67$
 $\Omega_v = 1.50$

$\phi_b = 0.90$
 $\phi_v = 1.00$

ASD $L_p = 5.55'$
 $L_r = 15.90'$ Choose section W16x40
 $Z_x = 73.00 > 72.14 \text{ in}^3$

Given $M_T = 180 \text{ FT.kips}$

We Can Use

W16x40

ASD

Table 1-1 (continued)

W-Shapes

Properties

Part 2 $\rightarrow r_y = 1.57''$



W16-W14

Nominal WT. lb/ft	Compact Section Criteria		Axis X-X				Axis Y-Y				r_{ts} in.	h_o in.	Torsional Properties	
	$\frac{b_f}{2t_f}$	$\frac{h}{t_w}$	I in. ⁴	S in. ³	r in.	Z in. ³	I in. ⁴	S in. ³	r in.	Z in. ³			J in. ⁴	C_w in. ⁶
50	5.61	37.4	659	81.0	6.68	92.0	37.2	10.5	1.59	16.3	1.89	15.7	1.52	2270
45	6.23	41.1	586	72.7	6.65	82.3	32.8	9.34	1.57	14.5	1.87	15.5	1.11	1990
40	6.93	46.5	518	64.7	6.63	73.0	28.9	8.25	1.57	12.7	1.86	15.5	0.794	1730
36	8.12	48.1	448	56.5	6.51	64.0	24.5	7.00	1.52	10.8	1.83	15.5	0.545	1460

$L_p = 42.386 \text{ ft}$ $r_y = 42.386 (1.57) = 66.546 = 5.55'$

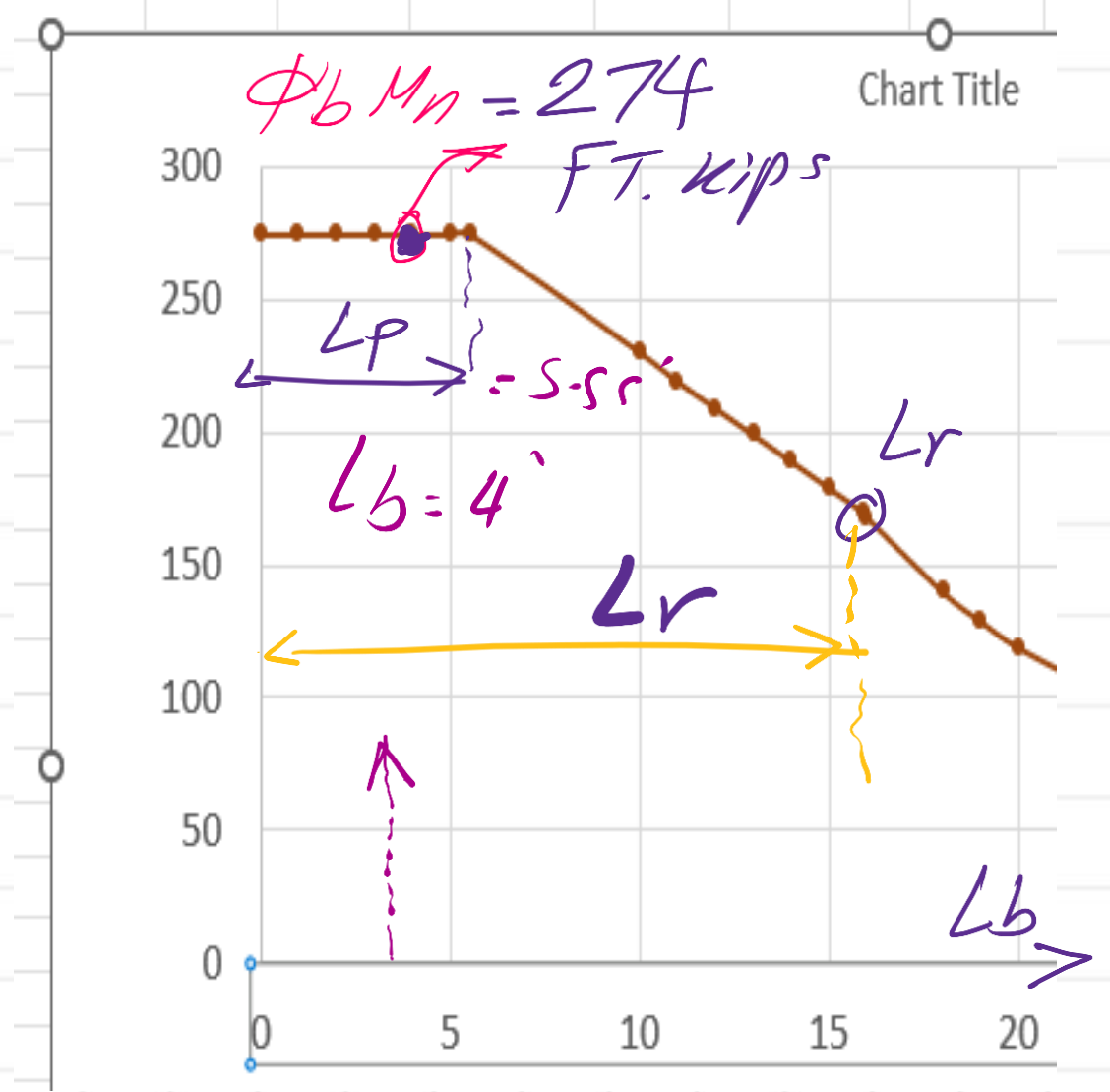
$L_b = 4' < 5.55'$ section is Compact¹²

Given $\frac{M_n}{L_b} = \frac{F_y Z_x}{L_b} = \frac{50 (73)}{1.67 (12)} = 182.14 \text{ FT.kips}$
 $> 180 \text{ FT.kips}$

LRFD

Excel graph - Part #1

length of bracing-Ft	Phi*Mn	Mn*(1/Ω)*1.00
0	274	182
1	274	182
2	274	182
3	274	182
4	274	182
5	274	182
5.55	274	182
10	230	153
11	219	146
12	209	139
13	199	132
14	189	126
15	179	119
15.895	170	113
16		

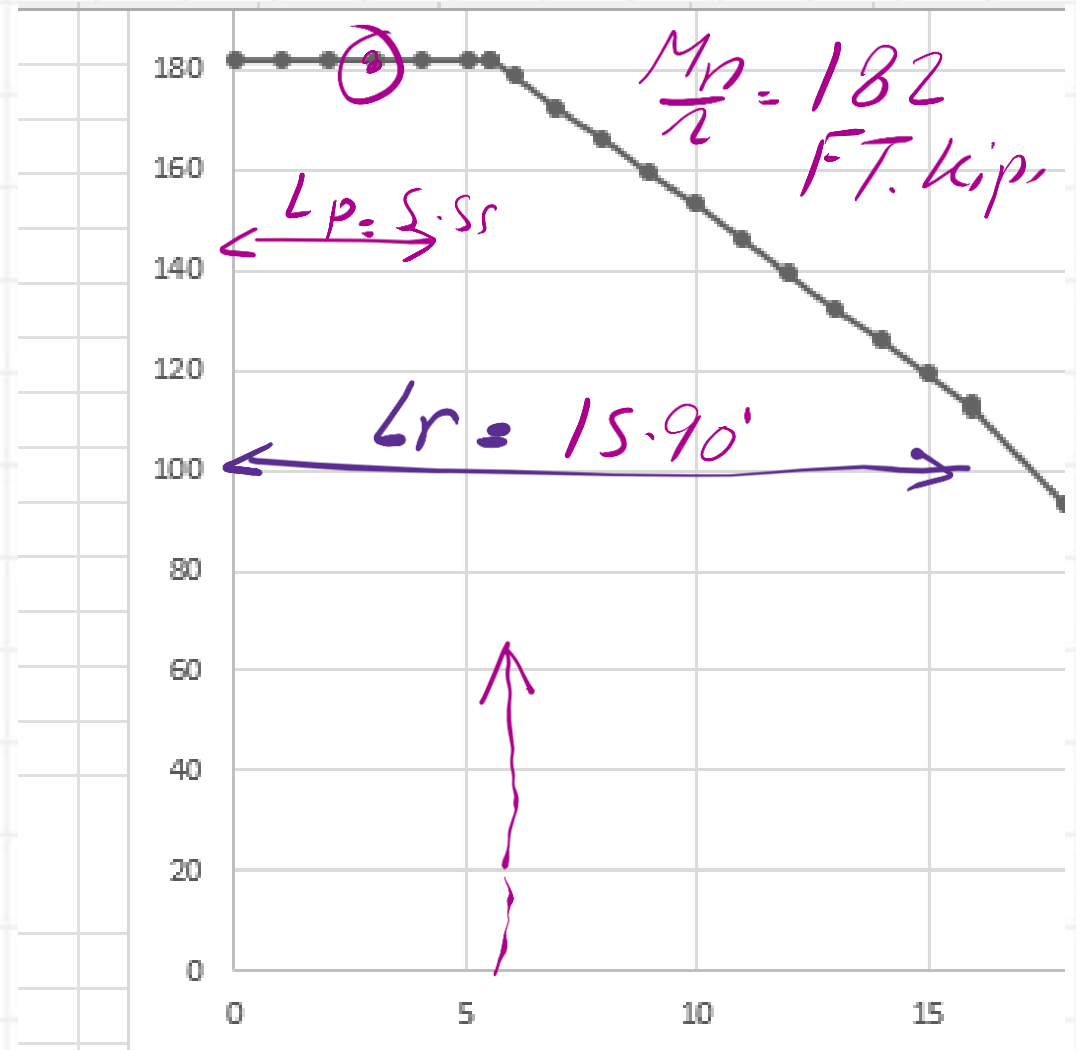


L_p

L_r

Excel graph # 1

$M_T = 180 \text{ FT. kip}$



length of bracing-Ft
0
1
2
3
4
5
5.55
10
11
12
13
14
15
15.895
16

Phi*Mn	Mn*(1/Ω)*1.00
274	182
274	182
274	182
274	182
274	182
274	182
274	182
274	182
274	182
274	182
230	153
219	146
209	139
199	132
189	126
179	119
170	113

L_p

L_r

LRFD Part-2 Select W section with min. allowable depth

$M_U = 270$

$F_y = 50 \text{ ksi}$ FT.kip.

$L_b = 4'$

Table 3-2 (continued)

W-Shapes

Selection by Z_x

Z_x

$L_p = 9.08'$
 $L_r = 36.60'$

Shape	Z_x	M_{px}/Ω_b	$\phi_b M_{px}$	M_{rx}/Ω_b	$\phi_b M_{rx}$	BF/Ω_b	$\phi_b BF$	L_p	L_r	I_x	V_{rx}/Ω_v	$\phi_v V_{rx}$
		kip-ft	kip-ft	kip-ft	kip-ft	kips	kips				kips	kips
	in. ³	ASD	LRFD	ASD	LRFD	ASD	LRFD	ft	ft	in. ⁴	ASD	LRFD
W10x60	74.6	186	280	116	175	2.54	3.82	9.08	36.6	341	85.7	129
W16x40	73.0	182	274	113	170	6.67	10.0	5.55	15.9	518	97.6	146
W12x50	71.9	179	270	112	169	3.97	5.98	6.92	23.8	391	90.3	135
W8x67	70.1	175	263	105	159	1.75	2.59	7.49	47.6	272	103	154
W14x43	69.6	174	261	109	164	4.88	7.28	6.68	20.0	428	83.6	125
W10x54	66.6	166	250	105	158	2.48	3.75	9.04	33.6	303	74.7	112

Select W10x60 → depth 10" < 16" but has higher $Z_x = 74.60 \text{ inch}^3$
 From Table $\phi_b M_n = 280 \text{ FT.kip} > 270 \text{ FT.kip}$

Part-2 Select W section with min. allowable depth

$$M_T = 180$$

$$F_y = 50 \text{ ksi } FT. \text{ kip.}$$

$$L_b = 4'$$

Table 3-2 (continued)

W-Shapes

Selection by Z_x

Z_x

Shape	Z_x	M_{px}/Ω_b	$\phi_b M_{px}$	M_{rx}/Ω_b	$\phi_b M_{rx}$	BF/Ω_b	$\phi_b BF$	L_p	L_r	I_x	V_{rx}/Ω_v	$\phi_v V_{rx}$
		kip-ft	kip-ft	kip-ft	kip-ft	kips	kips				kips	kips
	in. ³	ASD	LRFD	ASD	LRFD	ASD	LRFD	ft	ft	in. ⁴	ASD	LRFD
W10x60	74.6	186	280	116	175	2.54	3.82	9.08	36.6	341	85.7	129
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W14x43	69.6	174	261	109	164	4.88	7.28	6.68	20.0	428	83.6	125
W10x54	66.6	166	250	105	158	2.48	3.75	9.04	33.6	303	74.7	112

Select W10x60 → depth 10" < 16" but has higher $Z_x = 74.60 \text{ inch}^3$
 above W16x40
 From Table $M_{px}/\Omega_b = 186 \text{ FT. kip.} > 180 \text{ FT. kip.}$

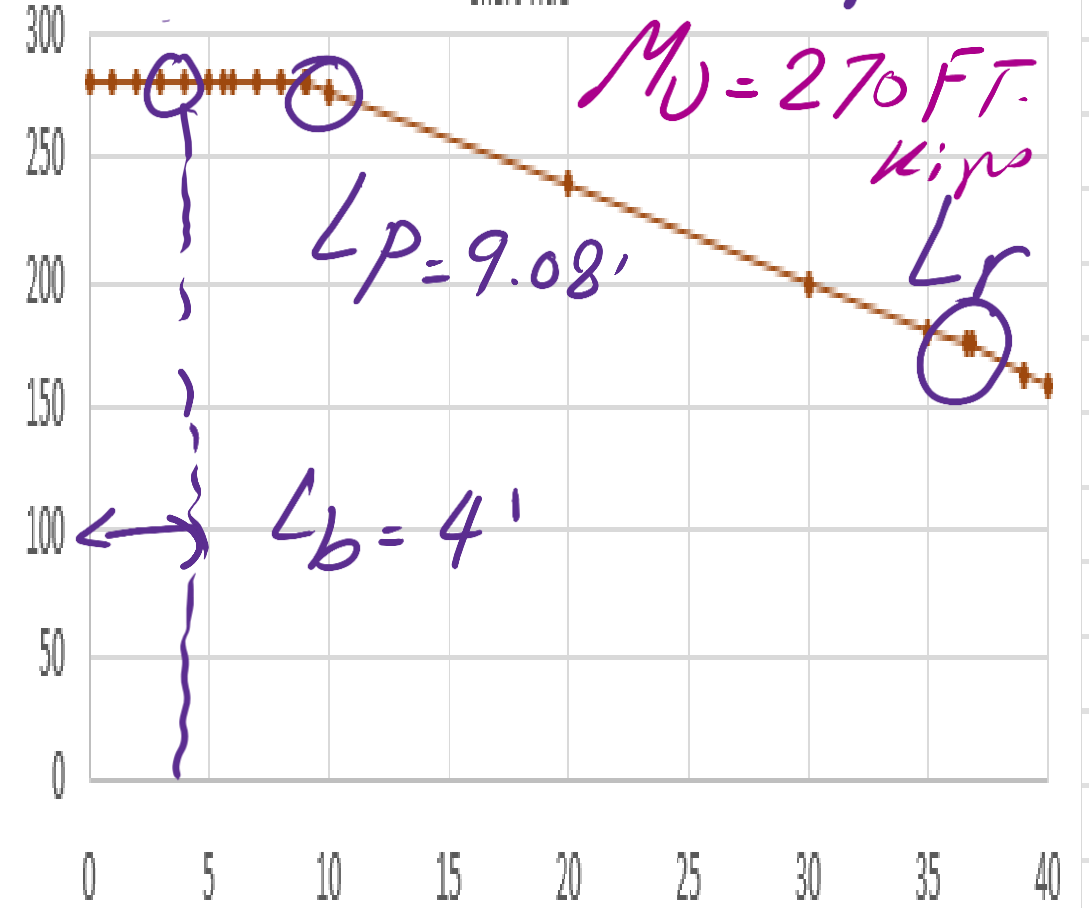
LRFD

length of bracing-Ft
0
1
2
3
4
5
5.58
6
7
8
9
9.08
10
20
30
35
36.614

Phi*Mn	Mn*(1/lambda)*1.00
280	186
280	186
280	186
280	186
280	186
280	186
280	186
280	186
280	186
280	186
280	186
280	186
280	186
280	186
280	186
276	184
239	159
200	133
181	120
176	117

Excel sheet # 2

$\phi M_n = 280 \text{ FT. kips}$



L_b

L_p

L_r

Case # 2

ASD

length of bracing-Ft
0
1
2
3
4
5
5.58
6
7
8
9
9.08
10
20
30
35
36.614

Phi*Mn	Mn*(1/phi)*1.00
280	186
280	186
280	186
280	186
280	186
280	186
280	186
280	186
280	186
280	186
280	186
280	186
280	186
280	186
276	184
239	159
200	133
181	120
176	117

W10 x 60 Section

